PLANNING and ENVIRONMENT ACT
Warnambool PLANNING SCHEME
<b>PERMIT NO. PA2201869</b>
MODIFIED ENDORSED PLAN
// Sheet 1 of 23
$V V \sim I$
Signed: for
MINISTER FOR PLANNING
Date: 06 March 2024



## EMMANUEL COLLEGE

## STORMWATER MANAGEMENT PLAN

CRAWLEY STREET WARRNAMBOOL

> 28/02/2024 Version: 3

For: Emmanuel College



### **Document History**

#### Rev. No. Description of Revision

- 1 First Report
- 2 Amended appendix
- 3 Amended terminology

Disclaimer:

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### Attachments

	<u>Sheet No.</u>	Description
	SW1	Stormwater Catchment Plan
	SW2	Site Soakage Test
	SW3	Soak Test Calculations
	SW4	Stormwater Storage & Discharge Calculation
	SW5	Orifice Plate Calculations
	SW6	MUSIC Results
2022.152	C-12, C-21 & C-32	Year 9 Centre Stormwater Layout Plan, Long section & Details

### References

The CSE Group Consulting Engineers

– 2022.152 Emmanuel College Year 9 Centre, Stormwater Management Plan, 28-09-2022



SED PLAN

for

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#### 1.0 INTRODUCTION

A stormwater management plan was previously produced for the proposed year 9 centre off Crawley Street (CSE#2022.152). Recently, Emmanuel College was able monostic and the land ling known as 15 Crawley Street and are looking to apply for planning approvals formastafficerpark2004 this land and the land on 13 Crawley Street for teachers and staff at the year 9 centre. This stormwater report will review the proposed stormwater management system for the Year 9 Centre and adjust it to account for the additional land that is being developed into carparks and ensure the stormwater discharge from the subject site is compliant with Council requirements and BPEM guidelines. Whilst subject to further planning approval, the stormwater infrastructure would be designed to allow for any future development such as hardstands and carparking areas. Stormwater detention is investigated for the proposed site and carpark to limit the adverse effects of stormwater on downstream properties up to the 1% AEP rainfall event. Stormwater treatment is designed to meet BPEM guidelines.

#### 2.0 **EXISTING CONDITIONS**

The proposed site for the year 9 centre and carpark falls northeast onto Crawly Street. There is an existing Council underground drainage pipe approximately 60m east of the proposed site which runs north along Ardlie Street and discharges into Russell's Creek.

Ground contours indicate that overland flow paths from the proposed site do not reach the Russell Creek and instead become land locked in a low-lying area on the block surrounded by Conns Lane. Ardlie Street, Wentworth Street and Barbers Lane. Water sitting in this location would likely pond in a major event before infiltrating into the ground.

#### 3.0 **DESIGN REQUIREMENTS**

It is anticipated that the proposed site will be required to meet the following conditions:

- Underground drainage to the legal point of discharge.
- On-site detention of stormwater to limit outflows to pre-development conditions in a 1% AEP rainfall event (due to the lack of overland outfall).
- All stormwater to be designed in accordance with the Infrastructure Design Manual & AS/NZS 3500.
- Meet Best Practice Environmental Guidelines:
  - 80% Reduction in Suspended Solids
  - 45% Reduction in Total Phosphorus
  - 45% Reduction in Total Nitrogen
  - 70% Reduction in Gross Pollutants
  - Maintain flows at 1.5 year ARI pre-development levels



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### 4.0 REVIEW, DESIGN & DISCUSSION

The stormwater management system that was proposed for the Year **Gigmentre as part of the for** 2022.152 SWMP incorporates a stormwater detention tank to attenuate flows from ground water runoff and a raingarden treatmentiareau to treat stormwater to BPEM. With the inclusion of more impervious surface from the proposed carpark the same elements will be utilised, though their sizes will be adjusted to suit the proposed conditions.

Stormwater attenuation is designed to limit flows to pre-developed conditions in a 1% AEP rainfall event to prevent exacerbation of flooding in the land locked area to the east.

Stormwater quality improvements are designed to meet or exceed BPEM guidelines.

#### 4.1 Catchments & Coefficient of Runoff for Development

The Coefficient of Runoff of the development has been calculated in accordance with the infrastructure design manual and the following:

- Landscaped Areas 0.25
- Low Density Residential Zone > 2ha 0.3
- Commercial Zones 0.9
- Paved Areas 0.95

Catchments were determined based on landform and surface finishes. Layouts of the catchments are provided in the appendix with results summarised below:

Catchment	Catchment	Area	Runoff
No.	Name	( <i>m</i> ²)	Coefficient
5	Crawley Yard & Carpark	1234	0.95
8	West of Footpath Landscaping	324	0.25
9	North of Year 9 Centre Landscaping	1252	0.25
10	South of Gym Landscaping	351	0.25
11	Year 9 Centre & Entrance Courtyard	3538	0.95

#### 4.2 Pre-development Runoff

The Rational Method is used to determine the pre-development runoff for a 1% AEP event with a **10 minute** time of concentration and a **0.3** runoff coefficient. Allowable runoff for the new development was calculated to be:

• Year 9 Centre & Crawley Street Carpark – 70.3 litres / second

#### 4.3 Roof Stormwater Collection

Rainfall collected from the roof of the year 9 building has been addressed in the previous report. Please refer to the 2022.152 SWMP for further details.

#### 4.4 Infiltration during 1% AEP Storm Event

The hydraulic conductivity of the soil surrounding the matrix tank was determined in a single bore hole in close proximity to the proposed soakage area. During excavation of the bore hole the following soil profile was discovered:

Emmanuel College Stormwater Management Plan Update for Crawley Street Development



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- Topsoil to 200mm depth
- Medium grain, brown sand to 500mm depth
- Medium to coarse grain, white sand to 780mm depth
- Bore hole refusal at 780mm

The bore hole was pre-soaked with water prior to the soakage test to ensure the test reflected the soils capacity to infiltrate water when wet. A soakage test was constructed in the coarse sand and tested using the constant head method. Test details are included on sheet SW2 and the following results were determined:

- Soak Test 1 180mm/hr
- Soak Test 2 120mm/hr
- Soak Test 3 82mm/hr

It should be noted that previous soil tests on the subject site show that there is a layer of sandstone over dune sands which have high infiltration rates. Previous soakage tests conducted on the site that have managed to get below the sandstone have resulted in higher rates of hydraulic conductivity. For the purposes of this report, the average hydraulic conductivity of 137mm/hr was adopted for the design of the matrix tank as per sheet SW3. The proposed 5.5m x 2.4m x 1.5m soakage pit is expected to soak 0.75 L/s.

#### 4.5 Storage for 1% AEP Storm Event

The required storage volume for the proposed development is shown on sheet SW3 to limit development flows to peak pre-development flows for a 1% AEP storm event. Storage volumes are:

• Year 9 Centre – 65.0 kilolitres

It is proposed that the previously recommended 100 kilolitre rainwater detention / retention tank be retained to limit flows from the roof (see prior 2022.152 SWMP for further details). The underground infiltration tank was upsized to a 17.8 kilolitre to limit flows from surface water. The underground tank limits discharge flows to pre-development levels through both storing excess stormwater for later release and infiltrating some stormwater into groundwater stores. The raingarden also provides some storage capacity but is primarily used to improve stormwater discharge quality. For details refer to sheet C-12.

#### 4.6 Discharge for 1% AEP Storm Event

Discharge rates are to be limited to the 1% AEP rainfall event using an orifice plate located at the outlet of the underground infiltration tank. The orifice calculation can be found on sheet SW4 and is summarised below:

• Year 9 Centre, underground tank – 150mm dia. @ bottom of tank

The orifice for the above ground tanks was previously calculated in the 2022.152 report, please refer to this report for above ground tank details.

#### 4.7 Proposed Storm Water Connection Points

Proposed discharge points for each allotment are:

• Year 9 Centre – Stormwater pit at front of 5 Crawley Street

Pipe size for the connection between the raingarden and council drainage assets is 150mm uPVC.

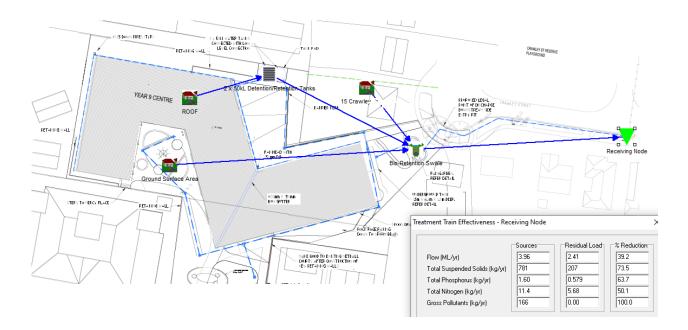


#### 4.8 Overland Flow Path

In storm events where the drainage system is exceeded, site stormwater will surcharge from the underground storage pit and head east down Crawley Street and south down Ardlie Street before ending up in the low-lying area between Conns Lane, Ardlie Street, Wentworth Street and Barbers Lane. Flows in a 1% AEP event will be limited to pre-development levels ensuring flooding in this area is not exacerbated.

#### 4.9 Stormwater Quality Treatment

Upon review of the previous system (2022.152 SWMP) it was found that the proposed raingarden was able to be reduced in size and still meet BPEM. The proposed stormwater detention system & raingarden was modelled in MUSIC to determine the effectiveness of these treatments at reducing stormwater pollutants from entering the stormwater network. A summary of results is presented below as a screenshot and a table with detailed results provided in the appendix. The quality of stormwater leaving the site is expected to exceed BPEM requirements.



	Source	<b>Residual Loads</b>	% Reduction	BPEM % Targets
Flow (ML/yr)	3.96	2.41	39.2	-
Total Suspended Solids (kg/yr)	781	207	73.5	80
Total Phosphorus (kg/yr)	1.60	0.579	63.7	45
Total Nitrogen (kg/yr)	11.4	5.68	50.1	45
Gross Pollutants (kg/yr)	166	0.00	100.0	70

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### 5.0 CONCLUSION & RECOMENDATIONS

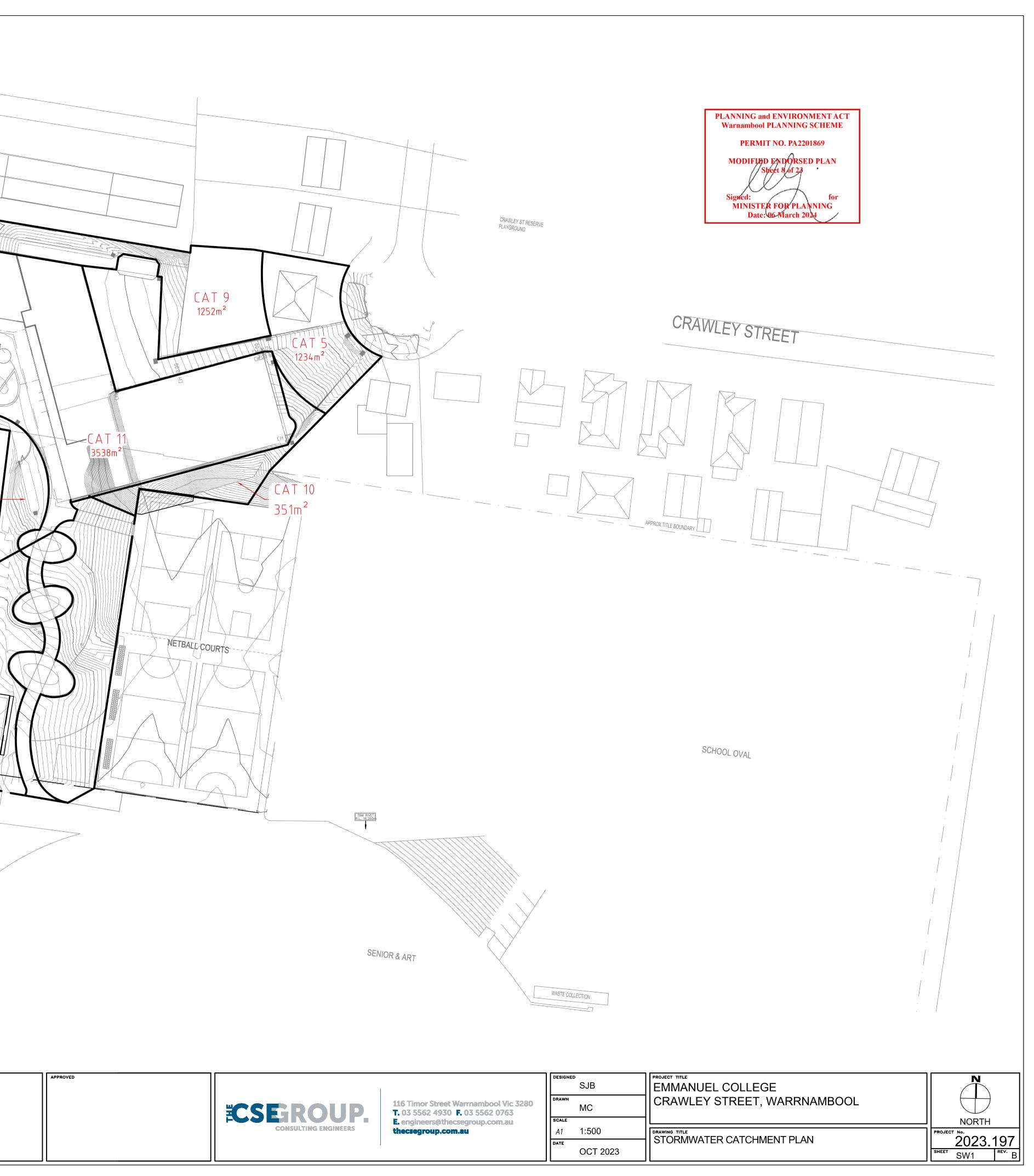
The adjusted Stormwater Management Plan for the proposed Year 9 Centre Development, requires the following works:

- Year 9 Centre (refer previous 2022.152 SWMP):
  - Connect all roof water down pipes by charged 125/150/225mm diameter stormwater pipe to a 100 kilolitre rainwater detention / retention tank.
  - Install 125mm diameter mid-level outlet in rainwater detention tank and connect to bioretention swale.
- Changes to Underground Drainage System to allow for potential future impervious surfaces:
  - Install 5m long 2m wide bioretention swale between outlet of underground drainage network and underground storage network for stormwater quality treatment.
  - o Install underground 17.8 kilolitre detention / infiltration tank as per attached details.
  - Connect underground pit with 150mm diameter uPVC to legal point of discharge.

--- END OF REPORT ---

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YEAR 9 CENTRE \_\_\_\_\_ CAT'8 324m² MERCY PLACE AGED CARE FACILITY TBM SPIK R.L. 21.47 FUTSAL COURT G BLOCK LAYOUT PLAN SCALE 1:500 28/02/2024 30/11/2023 AMENDED PROJECT TITLE B PRELIMINARY ISSUED FOR COMMENT А REV. DATE DESCRIPTION



APPROVED			DESIGNE	SJB	
	<b>CSEGROUP</b>	116 Timor Street Warrnambool Vic 3280 T. 03 5562 4930 F. 03 5562 0763	DRAWN	МС	C
	CONSULTING ENGINEERS	E. engineers@thecsegroup.com.au thecsegroup.com.au	scale A1	1:500	
			DATE	OCT 2023	S1

F:\archive\2023\2023.197 EC 15 Crawley St\C3D\2023.197 Catchments.dwg, SW1, 28/02/2024 11:15:46 AM, stephenb, AutoCAD PDF (High Quality Print).pc3, ISO full bleed A1 (841.00 x 594.00 MM), 1:1



116 Timor Street Warrnambool Vic 3280 **T.** 03 5562 4930 **F.** 03 5562 0763 **ABN.** 59 077 506 506 **E.** engineers@thecsegroup.com.au **thecsegroup.com.au** 

Project: 2023.197	
Address: 15 CRAWLEY STREET, W'B	OCL Date: 29/8/23 Operators: SB, MC
	Recent Weather: # FAIR, SOME DRIZZLE
Test: SOIL PERMEABILITY WORKSHEET	Site Conditions: SUNNY
Auger: Manual auger Dechanical Auger	
Method: 🗹 Constant Head 🛛 🗆 Falling Head	Site Diars
Comments:	Site Plan:
Soak Well Properties	
Daimeter Auger Hole: 0.055 m	CRAWLEY ST
Diameter Tube: 0.035 m	
Diameter inner tube: 0.009 m	
Hole Depth at Start of Test D <sub>s</sub> : $780$ mm	
Apparatus Depth D <sub>a</sub> : $250$ mm	n 4,5m
Hole depth at end of test D <sub>e</sub> :	n 4.5m TO FEETPART X
Test Depth (= $D_e$ - $D_a$ ): 530 mm	DAI VEAK
	8,00 9
Bore Hole Log	FROM CENTRE
Start End Description	TITLE
O 200 BLACK TOPSOIL	PLANNING and ENVIRONMENT ACT
200 500 RED SAND FINE GRAN	Warnambool PLANNING SCHEME
500 780 Mort YELLOW SAND	RECANDSTOLLE
780 BH Refusal	PERMIT NO. PA2201869
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Test 1 SHIELD

 $\sim 1$ 

Reading	Time	Interval	
(mm)	(m:ss)	(s)	
100			
200			
300	27		]
400	44	17	
500	1:01	+617	
600	1:20	19	]
700	1:40	20	]
800	2:02	22	
900	2:26	24	]
1000	2:50	24	]
1100			]
1200			]
1300			]
			_
100m	m Interval:	20	s
Pe	rmeability:	180	]mm/h

Test 2 - S HIELD

Reading	Time	Interval
(mm)	(m:ss)	(s)
100		
200	1:26	
300	1:57	31
400	2:25	28
500	2:54	29
600	2:24	30
700	3:53	29
, 800	4:23	30
900	4:53	30
1000	5:24	31
1100		
1200		
1300		

100mm Interval: 30 s Permeability: 120 mm/h

Interval Reading Time (mm) (m:ss) (s) 100 200 300 400 1:54 500 2:36 600 3:17 700 42 3:59 800 4:42 43 900 1000 1100 1200 1300 100mm Interval: 42 S Permeability: 82 mm/h

Date: 06 March 202

Test 3



29-Aug-23 SB, MC

Date:

By:

#### 2023.197 Emmanuel College 15 Crawley St 15 Crawley Street, Warrnambool, 3280

#### Soakage Testing

Soak Test 1

Location:	See Field Sheet
Method:	СНТ
Site Conditions:	Grassed area. Slight fall to SE
<b>Recent Weather:</b>	Warm, fine for several days
Comments:	Hole presoaked for several minutes.

Soak Well Properties		Round Well	
Diameter (auger hole	0.055	m	
Diameter (tube)	0.035	m	
Diameter (inner tube	0.009	m	
Depth	0.530	m	h
Vol.	1.26	L	-

#### **Constant Head Test**

 $k_{\rm h} = Q / [\pi r_{\rm o}^2 + 2\pi r_{\rm o} h]$ 

$$Q = V_c / t_c$$

Test 1								
Water	Time		Reading	Discharge	k <sub>h</sub> Soil Conductivity		Comments	
Source	(min)	(sec)	(m)	(m <sup>3</sup> /s)	m/sec	mm/hr		
	0	27	0.3					
<mark>∕</mark>	0	44	0.4	5.29E-06	5.63E-05	202.5		
р Ц	1	1	0.5	5.29E-06	5.63E-05	202.5		
sse	1	20	0.6	4.73E-06	5.03E-05	181.2		
- ¥	1	40	0.7	4.49E-06	4.78E-05	172.1		
ad	2	2	0.8	4.08E-06	4.35E-05	156.5		
н	2	26	0.9	3.74E-06	3.98E-05	143.4		
Constant Head Assembly	2	50	1	3.74E-06	3.98E-05	143.4		
nsta								
Ö								

mm/hr

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Test 2							
Water	Time		Reading	Discharge	k <sub>h</sub> Soil C	onductivity	Comments
Source	(min)	(sec)	(m)	(m <sup>3</sup> /s)	m/sec	mm/hr	
	1	26	0.2				
≥	1	57	0.3	2.90E-06	3.08E-05	111.1	
Assembly	2	25	0.4	3.21E-06	3.42E-05	123.0	
ser	2	54	0.5	3.10E-06	3.30E-05	118.7	
As	3	24	0.6	2.99E-06	3.19E-05	114.8	
be	3	53	0.7	3.10E-06	3.30E-05	118.7	
Head	4	23	0.8	2.99E-06	3.19E-05	114.8	
t	4	53	0.9	2.99E-06	3.19E-05	114.8	
stant	5	24	1	2.90E-06	3.08E-05	111.1	



#### 2023.197 Emmanuel College 15 Crawley St 15 Crawley Street, Warrnambool, 3280

#### Soakage Testing



Average k<sub>h</sub> 116 m

mm/hr

Water	٦	Гime	Reading	Discharge	k <sub>h</sub> Soil Conductivity		Comments
Source	(min)	(sec)	(m)	(m <sup>3</sup> /s)	m/sec	mm/hr	
	1	54	0.4				
≥	2	36	0.5	2.14E-06	2.28E-05	82.0	
qu	3	17	0.6	2.19E-06	2.33E-05	84.0	
ser	3	59	0.7	2.14E-06	2.28E-05	82.0	
As	4	42	0.8	2.09E-06	2.22E-05	80.1	
Constant Head Assembly							
Ĕ							
t							
sta							
Suo							
Ō							

Average k<sub>h</sub> 82 mm/hr

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PLANNING and ENVIRONMENT ACT Warnambool PLANNING SCHEME

116 Timor Street Warrnambool Vic 3280 **T**, 03 5562 4930 **F**. 03 5562 0763 ABN. 59 077 506 506 E. engineers@thecsegroup.com.au Bredsegroup.com.au Bredsegroup.com.au Bredsegroup.com.au () 1869

75% Impervious

2022.152 Emmanuel College - Year Nine Centre 140 Botanic Road, Warrnambool, 3280

#### Storage & Infiltration Calculation

CONSULTING ENGINEERS

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Site Characteristics	Area (m²)	Runoff Coefficient	C x A
CAT 11 Year 9 Centre and yard	3538	0.95	3361.1
CAT 5 Crawley yard & carpark	1234	0.95	1172.3
CAT 9 North grass	1252	0.25	313
CAT 8 West of footpath grass	324	0.25	81
South east of gym	351	0.25	87.75
Totals (m <sup>2</sup> )	6699		5015

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Predevelopment Runoff Predev. Runoff Coefficient - C 0.3 Find CA (Total Area by C) 2009.7 m<sup>2</sup> Storm Event Time of Concentration 1% AEP 10 mins Warrnambool Locality Code Intensity - I Runoff Q = CAI / 3600 WARR 126.0 mm/hr 70.3 L/s

WSUD Measures				Effective	Storage
			1	nfiltration Area	Volume
				(m²)	(m³)
Rainwater Tank					
Number of tanks	1	Height (m)	2.18		
Detention Volume	50 m <sup>3</sup>	Diameter (m)	7.642		
Retention Volume	50 m <sup>3</sup>				
% retention for private use	50%	Vol = n * (pi * D^2/4	* h) * (1-percentage)		50.00
Underground Matrix Pit					
Length - L	5.5 m	Base Infiltration Area	$A_{inf} = WxLx(1-F_b)$	13.20	
Width - W	2.4 m	Perimeter Infiltration	Area = 0.5*P*D	11.85	
Depth of Storage - d	1.5 m				
Infiltration Blockage Factor - F <sub>b</sub>	0%	Storage Volume			17.82
Storage Porosity P <sub>s</sub>	90%	$S_B = L \times W$	/ x d x P <sub>s</sub> + (LxWxd <sub>bed</sub> )		
Infiltration Bed Depression dbed	0 m				
Swale					
Length - L	5 m	Base Infiltration Area	$A_{inf} = WxLx(1-F_b)$	10.00	
Width - W	<mark>2</mark> m	Perimeter Infiltration	Area = 0.5*P*D*(1-F <sub>b</sub>	4.20	
Depth of Storage - d <sub>i</sub>	0.6 m	Storage Volume - ir	filtration media		1.20
Infiltration Blockage Factor - Fb	0%	$S_i = L \times W$	x d <sub>i</sub> x P <sub>s</sub>		
Storage Porosity P <sub>s</sub>	20%	Basin Vol. S <sub>a</sub> = L ((V	V d <sub>w</sub> ) + ((6 d <sub>w</sub> ^2)/2))		2.60
Infiltration Bed Depression d <sub>w</sub>	0.2 m				
		TOTALS		39.25	71.62

Soil Characteristics						
Soil K <sub>h</sub>	137	mm/hr	Infiltration Rate*	7.49E-04	m³/s	
	3.81E-05	m/s		0.749	L/s	
Moderating Factor - U	0.5	Sandy soil				

Equal to Predevelopment Runoff Conditions (unless no connection available)

Allow Piped Outflow Q<sub>p</sub> 1% AED Dainfall Event 70.3

L/s

Time	Rainfall	Volume In	Piped flow	Nett Inflow Vol.	Soakage Out	Storage Volume	Percentage of	Storage are
(Duration) min	Intensity	CAID/60.000			[A <sub>eff</sub> ] Uk <sub>h</sub> t	Required	storage provided	adequate
	,						<b>.</b> .	adequate
D	mm/hr	I <sub>v</sub> (m <sup>3</sup> )	$O_{p}$ (m <sup>3</sup> )	I <sub>v</sub> - O <sub>p</sub> (m3)	O <sub>s</sub> (m <sup>3</sup> )	$S_R = I_V - O_p - O_s (m^3)$	S <sub>T</sub> / S <sub>R</sub> %	
0	0	0	0	0	0	0		Yes
1	268.0	22.40	4.22	18.18	0.04	18.14	395%	Yes
2 3	202.0	33.77	8.44	25.33	0.09	25.24	284%	Yes
3	185.0	46.39	12.66	33.73	0.13	33.59	213%	Yes
4	173.0	57.84	16.88	40.96	0.18	40.78	176%	Yes
5	163.0	68.12	21.10	47.02	0.22	46.80	153%	Yes
10	126.0	105.32	42.20	63.11	0.45	62.67	114%	Yes
15	103.0	129.14	63.31	65.83	0.67	65.16	110%	Yes
20	86.3	144.27	84.41	59.86	0.90	58.96	121%	Yes
25	74.8	156.31	106	50.80	1.12	49.67	144%	Yes
30	66.1	165.75	127	39.14	1.35	37.79	190%	Yes
45	49.6	186.56	190	0.00	0.00	0.00		Yes
60	40.3	202.11	253	0.00	0.00	0.00		Yes
90	30.1	226.43	380	0.00	0.00	0.00		Yes
120	24.6	246.75	506	0.00	0.00	0.00		Yes
180	18.6	279.85	760	0.00	0.00	0.00		Yes
270	14.3	322.72	1139	0.00	0.00	0.00		Yes
360	12.0	361.09	1519	0.00	0.00	0.00		Yes
540	9.3	421.57	2279	0.00	0.00	0.00		Yes
720	7.8	471.22	3039	0.00	0.00	0.00		Yes
1080	6.1	547.96	4558	0.00	0.00	0.00		Yes
1440	5.0	603.02	6077	0.00	0.00	0.00		Yes
1800	4.3	643.95	7597	0.00	0.00	0.00		Yes
2160	3.7	675.24	9116	0.00	0.00	0.00		Yes
2880	3.0	717.37	12155	0.00	0.00	0.00		Yes
4320	2.1	761.90	18232	0.00	0.00	0.00		Yes
5760	1.6	779.96	24309	0.00	0.00	0.00		Yes
7200	1.3	794.40	30387	0.00	0.00	0.00		Yes
8640	1.1	801.62	36464	0.00	0.00	0.00		Yes
10080	1.0	96.40	4947	0.00	0.00	0.00		Yes



### 2023.197 Emmanuel College 15 Crawley St 15 Crawley Street, Warrnambool, 3280

#### **Orifice Plate**

 $Q = C_d a_o \sqrt{[2gh]}$  Rate of Discharge

C <sub>d</sub>	0.63		Coefficeint of Discharge
Q	70	L/s	Flow Rate
	0.07	m³/s	
h	1.5	m	Water height above orifice
g	9.81	m/s	m/s gravity
a <sub>o</sub>	0.020482	m²	Area of Orifice
Orifice Dia.	0.161	m	
	161	mm	

PLANNING and ENVIRONMENT ACT Warnambool PLANNING SCHEME
<b>PERMIT NO. PA2201869</b>
MODIFLED ENDORSED PLAN Sheet 13/of 28
Signed: for
MINISTER FOR PLANNING Date: 06-March 2024

File: 2023.197 Drainage (ID 255348).xlsm Sheet: Orifice Plate

## ESEGROUP. CONSULTING ENGINEERS

#### 116 Timor Street Warrnambool Vic 3280 T. 03 5562 4930 F. 03 5562 0763 ABN. 59 077 506 506 E. engineers@thecsegroup.com.au thecsegroup.com.au

MODIFIED EN

			/ `	^
Location	ROOF	Ground Surface Area	15 Crawley Carpaned	for
ID	1	5	MINE	TER FOR PLANNING ate: 06 March 2024
Node Type	UrbanSourceNode	UrbanSourceNode	UrbanSourceNode	ate: 00-wiarch 2024
Total Area (ha)	0.313	0.3	0.072	
Area Impervious (ha)	0.313	0.224421053	0.065024211	
Area Pervious (ha)	0	0.075578947	0.006975789	
Field Capacity (mm)	80	80	80	
Pervious Area Infiltration Capacity coefficient - a	200	200	200	
Pervious Area Infiltration Capacity exponent - b	1	1	1	
Impervious Area Rainfall Threshold (mm/day)	1	1	1	
Pervious Area Soil Storage Capacity (mm)	120	120	120	
Pervious Area Soil Initial Storage (% of Capacity)	30	30	30	
Groundwater Initial Depth (mm)	10	10	10	
Groundwater Daily Recharge Rate (%)	25	25	25	
Groundwater Daily Baseflow Rate (%)	5	5	5	
Groundwater Daily Deep Seepage Rate (%)	0	0	0	
Stormflow Total Suspended Solids Mean (log mg/L)	2.2	2.2	2.2	
Stormflow Total Suspended Solids Standard Deviation (log mg/L)	0.32	0.32	0.32	
Stormflow Total Suspended Solids Estimation Method	Stochastic	Stochastic	Stochastic	
Stormflow Total Suspended Solids Serial Correlation	0	0	0	
Stormflow Total Phosphorus Mean (log mg/L)	-0.45	-0.45	-0.45	
Stormflow Total Phosphorus Standard Deviation (log mg/L)	0.25	0.25	0.25	
Stormflow Total Phosphorus Estimation Method	Stochastic	Stochastic	Stochastic	
Stormflow Total Phosphorus Serial Correlation	0	0	0	
Stormflow Total Nitrogen Mean (log mg/L)	0.42	0.42	0.42	
Stormflow Total Nitrogen Standard Deviation (log mg/L)	0.19	0.19	0.19	]
Stormflow Total Nitrogen Estimation Method	Stochastic	Stochastic	Stochastic	
Stormflow Total Nitrogen Serial Correlation	0	0	0	

PLANNING and ENVIRONMENT ACT Warnambool PLANNING SCHEME

**PERMIT NO. PA2201869** 

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Location	ROOF	Ground Surface Area	15 Crawley Carpark
Baseflow Total Suspended Solids Mean (log mg/L)	1.1	1.1	1.1
Baseflow Total Suspended Solids Standard Deviation (log mg/L)	0.17	0.17	0.17
Baseflow Total Suspended Solids Estimation Method	Stochastic	Stochastic	Stochastic
Baseflow Total Suspended Solids Serial Correlation	0	0	0
Baseflow Total Phosphorus Mean (log mg/L)	-0.82	-0.82	-0.82
Baseflow Total Phosphorus Standard Deviation (log mg/L)	0.19	0.19	0.19
Baseflow Total Phosphorus Estimation Method	Stochastic	Stochastic	Stochastic
Baseflow Total Phosphorus Serial Correlation	0	0	0
Baseflow Total Nitrogen Mean (log mg/L)	0.32	0.32	0.32
Baseflow Total Nitrogen Standard Deviation (log mg/L)	0.12	0.12	0.12
Baseflow Total Nitrogen Estimation Method	Stochastic	Stochastic	Stochastic
Baseflow Total Nitrogen Serial Correlation	0	0	0
OUT - Mean Annual Flow (ML/yr)	2	1.54	0.423
OUT - TSS Mean Annual Load (kg/yr)	418	305	85.4
OUT - TP Mean Annual Load (kg/yr)	0.843	0.622	0.174
OUT - TN Mean Annual Load (kg/yr)	5.75	4.34	1.21
OUT - Gross Pollutant Mean Annual Load (kg/yr)	82.2	66.4	17.8
No Imported Data Source nodes			

PLANNING and ENVIRONMENT ACT Warnambool PLANNING SCHEME PERMIT NO. PA2201869 MODIFLED ENDORSED PLAN Sheet 15 of 28 Signed: for MINISTER FOR PLANNING Date: 06 March 2024

Sheet: Source nodes File: Music Results Formatter.xlsm Sheet: 2 of 7 Printed: 30/11/2023

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Location	2 x 50kL Detention/Retention Tanks	<b>Bio-Retention Swale</b>	Π
ID		2	3
Node Type	RainWaterTankNode	BioRetentionNode	
Lo-flow bypass rate (cum/sec)		0	0
Hi-flow bypass rate (cum/sec)	10	0 1	.00
Inlet pond volume		0	
Area (sqm)	5	0	5
Extended detention depth (m)		1	0
Permanent pool volume (cum)	5	0	
Proportion vegetated		0	
Equivalent pipe diameter (mm)	12	5	
Overflow weir width (m)	1	0	2
Notional Detention Time (hrs)	0.38	1	
Orifice discharge coefficient	0.	6	
Weir coefficient	1.	7 1	1.7
Number of CSTR cells		2	3
Total Suspended Solids k (m/yr)	40	0 80	00
Total Suspended Solids C* (mg/L)	1	2	20
Total Suspended Solids C** (mg/L)	1	2	
Total Phosphorus k (m/yr)	30	0 60	00
Total Phosphorus C* (mg/L)	0.1	3 0.	13
Total Phosphorus C** (mg/L)	0.1	3	
Total Nitrogen k (m/yr)	4	0 5	00
Total Nitrogen C* (mg/L)	1.	4 1	1.4
Total Nitrogen C** (mg/L)	1.	4	
Threshold hydraulic loading for C** (m/yr)	350	0	
Extraction for Re-use	On	Off	_
Annual Re-use Demand - scaled by daily PET (ML)		0	

**PERMIT NO. PA2201869** MODIFIED ENDORSED PLAN Signed: for MINISTER FOR PLANNING Date: 06 March 2024

#### PLANNING and ENVIRONMENT ACT Warnambool PLANNING SCHEME



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Location	2 x 50kL Detention/Retention Tanks	<b>Bio-Retention Swale</b>	Π
Constant Daily Re-use Demand (kL)	0.6		
User-defined Annual Re-use Demand (ML)	0		Ц
Percentage of User-defined Annual Re-use Demand Jan	8.33333333		
Percentage of User-defined Annual Re-use Demand Feb	8.33333333		
Percentage of User-defined Annual Re-use Demand Mar	8.33333333		
Percentage of User-defined Annual Re-use Demand Apr	8.333333333		
Percentage of User-defined Annual Re-use Demand May	8.333333333		
Percentage of User-defined Annual Re-use Demand Jun	8.333333333		
Percentage of User-defined Annual Re-use Demand Jul	8.333333333		
Percentage of User-defined Annual Re-use Demand Aug	8.333333333		
Percentage of User-defined Annual Re-use Demand Sep	8.333333333		
Percentage of User-defined Annual Re-use Demand Oct	8.333333333		
Percentage of User-defined Annual Re-use Demand Nov	8.333333333		
Percentage of User-defined Annual Re-use Demand Dec	8.33333333	,	
Filter area (sqm)			10
Filter depth (m)		0	0.3
Filter median particle diameter (mm)			2
Saturated hydraulic conductivity (mm/hr)		3	60
Voids ratio		0	0.3
Length (m)			
Bed slope			
Base Width (m)			
Top width (m)			
Vegetation height (m)			
Proportion of upstream impervious area treated			
Seepage Rate (mm/hr)	0	3	60
Evap Loss as proportion of PET	0		

PLANNING and ENVIRONMENT ACT Warnambool PLANNING SCHEME

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Location	2 x 50kL Detention/Retention Tanks	Bio-Retention Swale
Depth in metres below the drain pipe		0
IN - Mean Annual Flow (ML/yr)	2	3.74
IN - TSS Mean Annual Load (kg/yr)	418	502
IN - TP Mean Annual Load (kg/yr)	0.843	1.18
IN - TN Mean Annual Load (kg/yr)	5.75	9.55
IN - Gross Pollutant Mean Annual Load (kg/yr)	82.2	84.2
OUT - Mean Annual Flow (ML/yr)	1.78	2.41
OUT - TSS Mean Annual Load (kg/yr)	112	213
OUT - TP Mean Annual Load (kg/yr)	0.382	0.593
OUT - TN Mean Annual Load (kg/yr)	4.01	5.6
OUT - Gross Pollutant Mean Annual Load (kg/yr)	0	0
No Generic treatment nodes		

PLANNING and ENVIRONMENT ACT Warnambool PLANNING SCHEME
<b>PERMIT NO. PA2201869</b>
MODIFLED ENDORSED PLAN Sheet 18 of 28 Signed: for MINISTER FOR PLANNING
Date: 06 March 2024



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Location	Receiving Node
ID	4
Node Type	ReceivingNode
IN - Mean Annual Flow (ML/yr)	2.41
IN - TSS Mean Annual Load (kg/yr)	213
IN - TP Mean Annual Load (kg/yr)	0.593
IN - TN Mean Annual Load (kg/yr)	5.6
IN - Gross Pollutant Mean Annual Load (kg/yr)	0
OUT - Mean Annual Flow (ML/yr)	0
OUT - TSS Mean Annual Load (kg/yr)	0
OUT - TP Mean Annual Load (kg/yr)	0
OUT - TN Mean Annual Load (kg/yr)	0
OUT - Gross Pollutant Mean Annual Load (kg/yr)	0



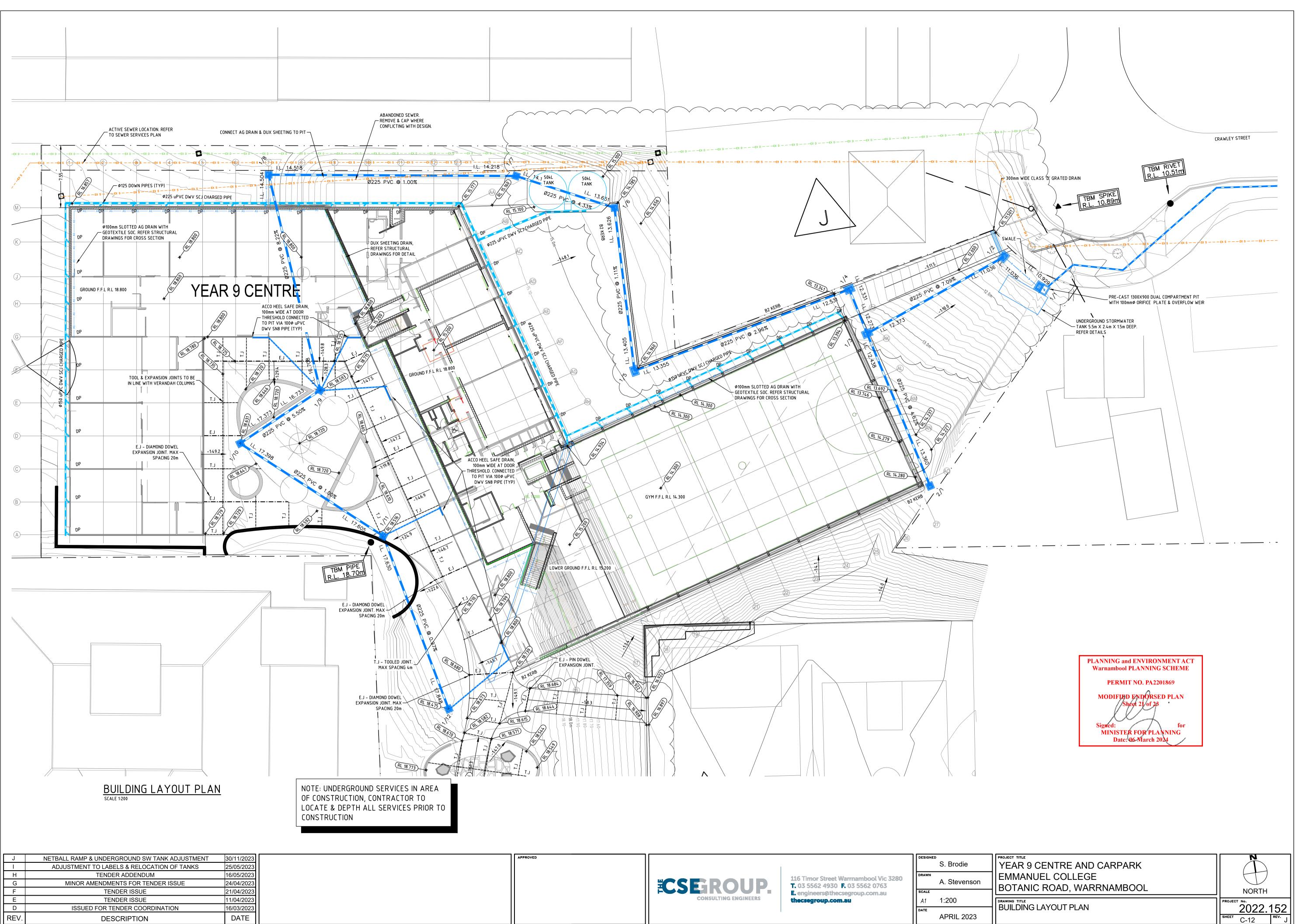
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Location	Drainage Link				
Source node ID	1	5	2	3	6
Target node ID	2	3	3	4	3
Muskingum-Cunge Routing	Not Routed				
Muskingum K					
Muskingum theta					
IN - Mean Annual Flow (ML/yr)	2	1.54	1.78	2.41	0.423
IN - TSS Mean Annual Load (kg/yr)	418	305	112	213	85.4
IN - TP Mean Annual Load (kg/yr)	0.843	0.622	0.382	0.593	0.174
IN - TN Mean Annual Load (kg/yr)	5.75	4.34	4.01	5.6	1.21
IN - Gross Pollutant Mean Annual Load (kg/yr)	82.2	66.4	0	0	17.8
OUT - Mean Annual Flow (ML/yr)	2	1.54	1.78	2.41	0.423
OUT - TSS Mean Annual Load (kg/yr)	418	305	112	213	85.4
OUT - TP Mean Annual Load (kg/yr)	0.843	0.622	0.382	0.593	0.174
OUT - TN Mean Annual Load (kg/yr)	5.75	4.34	4.01	5.6	1.21
OUT - Gross Pollutant Mean Annual Load (kg/yr)	82.2	66.4	0	0	17.8

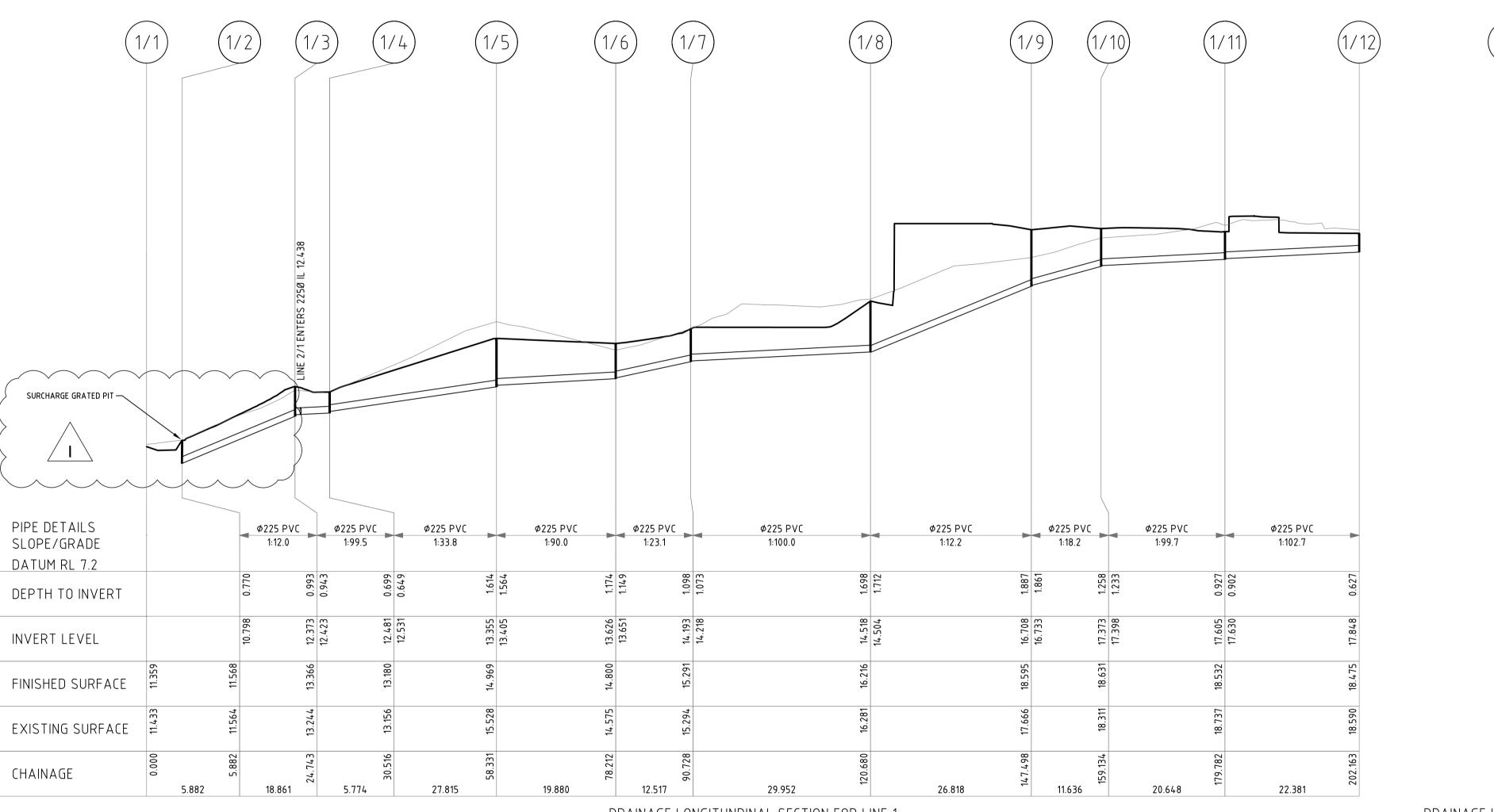
PLANNING and ENVIRONMENT ACT
Warnambool PLANNING SCHEME
<b>PERMIT NO. PA2201869</b>
MODIFLED ENDORSED PLAN
Sheet 20/of 28
Signed: for
MINISTER FOR PLANNING
Date: 06-March 2024

Sheet: 7 of 7 Printed: 30/11/2023



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APPROVED			DESIGNED	S. Brodie	P ,
	ESEGROUP.	116 Timor Street Warrnambool Vic 3280 T. 03 5562 4930 F. 03 5562 0763	DRAWN	A. Stevenson	
	CONSULTING ENGINEERS	E. engineers@thecsegroup.com.au thecsegroup.com.au	scale A1	1:200	
			DATE	APRIL 2023	ľ



Ι	NETBALL RAMP & UNDERGROUND SW TANK ADJUSTMENT	30/11/2023
Н	TENDER ADDENDUM	16/05/2023
G	MINOR AMENDMENTS FOR TENDER ISSUE	24/04/2023
F	TENDER ISSUE	21/04/2023
E	TENDER ISSUE	11/04/2023
D	ISSUED FOR TENDER COORDINATION	16/03/2023
REV.	DESCRIPTION	DATE

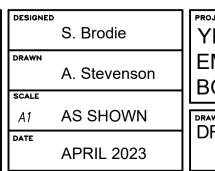
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DRAINAGE LONGITUNDINAL SECTION FOR LINE 1

APPROVED

DRAINAGE LONGITUNDINAL SECTION FOR LINE 2

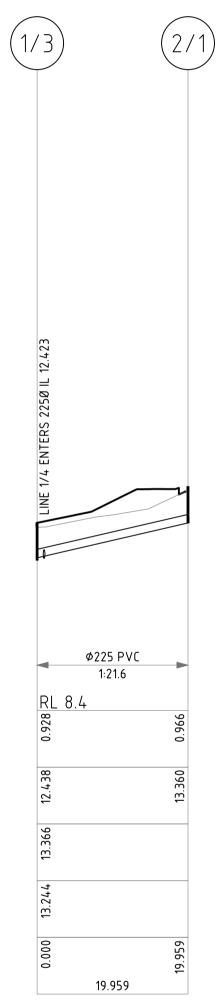


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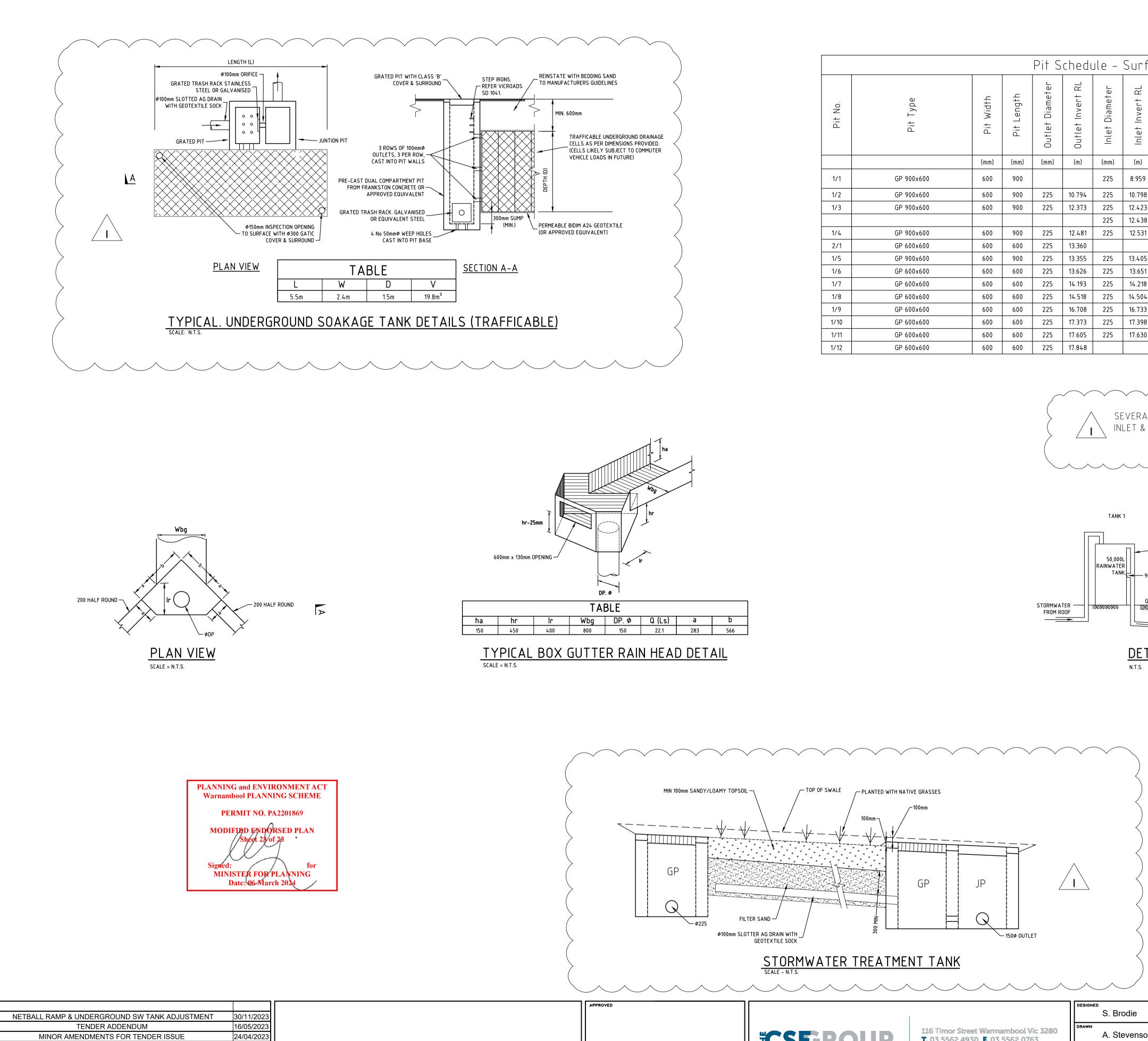
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PROJECT TITLE	
YEAR 9 CENTRE AND CARPARK	
EMMANUEL COLLEGE	
BOTANIC ROAD, WARRNAMBOOL	
DRAWING TITLE	PROJECT No.
DRAINAGE LONGITUDINAL SECTION	2022.152
	SHEET C-21 REV. I



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21/04/2023

11/04/2023

16/03/202

DATE

TENDER ISSUE

TENDER ISSUE

ISSUED FOR TENDER COORDINATION

DESCRIPTION

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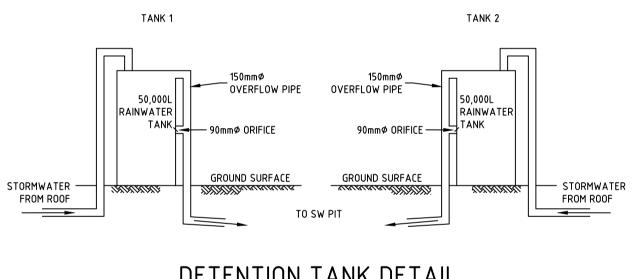
E |

D

REV.

	Pit Schedule – Surface SW											
Pit No.	Pit Type	Pit Width	Pit Length	Outlet Diameter	Outlet Invert RL	Inlet Diameter	Inlet Invert RL	Pit Depth	Pit Lid Level	Easting	Northing	Comment
		(mm)	(mm)	(mm)	(m)	(mm)	(m)	(m)	(m)	(m)	(m)	
1/1	GP 900x600	600	900			225	8.959	2.400	11.359	628984.04	5752236.062	PRE-CAST 1300X900 DUAL COMPARTMENT PIT
1/2	GP 900×600	600	900	225	10.794	225	10.798	0.678	11.472	628980.43	5752240.318	GRATED SURCHARGE PIT
1/3	GP 900×600	600	900	225	12.373	225	12.423	0.993	13.366	628962.58	5752233.911	
						225	12.438					
1/4	GP 900×600	600	900	225	12.481	225	12.531	0.699	13.180	628961.34	5752239.550	CLASS 'D' LID
2/1	GP 600x600	600	600	225	13.360			0.966	14.326	628967.03	5752214.456	
1/5	GP 900×600	600	900	225	13.355	225	13.405	1.614	14.969	628934.06	5752234.126	CLASS 'D' LID
1/6	GP 600×600	600	600	225	13.626	225	13.651	1.174	14.800	628934.94	5752253.987	CLASS 'D' LID
1/7	GP 600×600	600	600	225	14.193	225	14.218	1.103	15.296	628923.83	5752259.754	
1/8	GP 600x600	600	600	225	14.518	225	14.504	1.698	16.216	628894.32	5752264.864	
1/9	GP 600x600	600	600	225	16.708	225	16.733	1.887	18.595	628896.16	5752238.109	
1/10	GP 600x600	600	600	225	17.373	225	17.398	1.258	18.631	628885.53	5752233.373	
1/11	GP 600x600	600	600	225	17.605	225	17.630	0.927	18.532	628900.76	5752219.438	
1/12	GP 600x600	600	600	225	17.848			0.627	18.475	628905.10	5752197.480	





	APPROVED	ESEROUP. CONSULTING ENGINEERS	116 Timor Street Warrnambool Vic 3280 T. 03 5562 4930 F. 03 5562 0763	DESIGNE	S. Brodie	YEAR 9 CENTRE AND CARPARK	
				DRAWN	A Stevenson	EMMANUEL COLLEGE BOTANIC ROAD, WARRNAMBOOL	
				A1		DRAWING TITLE STORMWATER DETAILS	
				DATE	APRIL 2023	STORIVIVATER DETAILS	2022.152 SHEET C-32



## DETENTION TANK DETAIL