## SITE DRAINAGE MANAGEMENT REPORT

Site Address	Killesta Secondary College 427-441 Springvale Rd Springvale
Job Number	21/09
Date:	13/06/2021
Client:	Killesta Secondary College
Report By:	Martin Masina
Checked By:	Brian Bird
Appendices:	<ul> <li>A Killester College Senior Centre Functional Layout Plan</li> <li>B Kennedy Hall On-Site Stormwater Detention Computations</li> </ul>
	C Kennedy Hall On-Site Stormwater Music Modelling
	D Ann Street Carpark On-Site Stormwater Detention Computations
	E Ann Street Carpark On-Site Stormwater Storm Modelling

Revision B 21.06.2021

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ABN 90 633 978 346

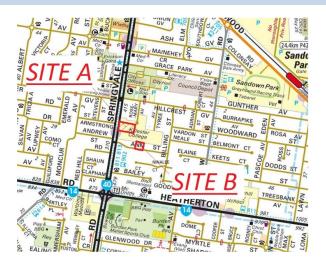
#### INTRODUCTION

This report will outline the drainage analysis for the development of the proposed extension to the Killesta Secondary Collage senior centre Kennedy Hall, and associated carpark and the construction of the additional Ann Street carpark. The report embraces a range of measures that are designed to mitigate the increase in stormwater runoff as well as to avoid the environmental impacts of the potential pollution threat to the runoff itself.

#### **PROJECT OBJECTIVES**

- 1) Minimise stormwater discharge off site from the increase in the impervious are of the proposed development.
- Minimising potable water demand by harvesting and storing stormwater in rainwater tanks for sanitary flushing of toilets.
- Treating stormwater on-site to improve the water quality and reduce the flow into the council's stormwater system.

#### SITE DESCRIPTION



The site lies on the east side of Springvale Road, Springvale and consists of an existing school hall, classrooms, amenities, and associated carpark. The site of the southern carpark is a collection of existing dwelling fronting the northern side of Ann Street Nos. 5 &7 awaiting demolition.

#### SITE TOPOGRAPHY

The topography of both sites is fully developed sites with the natural fall of the land being in a south westerly direction away from the developments.

#### SITE STATISTICS.

<u>Kennedy Hall – Site A</u> Site Area = 4170m2 Existing Impervious Area = 2870m2 Total Site Coverage70% Coefficient C=0.70

Proposed Impervious Area = 3962m2 Total Site Coverage = 95% Coefficient C=0.95 <u>Ann Street Carpark – Site B</u> Site Area = 1070m2 Existing Impervious Area = 792m2 Total Site Coverage = 74% Coefficient C=0.74 Proposed Impervious Area = 1016m2 Total Site Coverage = 95% Coefficient C=0.95

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#### LEGAL POINTS OF DISCHARGE

#### Kennedy Hall

The discharging of stormwater from the existing Kennedy Hall redevelopment will be into an existing side entry pit located on the eastern side of Springvale Road north of the existing school entrance pending town planning approval. (TBC)



#### Ann Street Carpark

The discharging of stormwater from the southern carpark will be into an existing side entry pit located on the north frontage of No.3 Ann Street just west of the site. This will include a short length of outfall drain located behind the existing kerb and channel.



#### WSUD TREAMENT MEASURE OPTIONS OPEN TO THE COLLEGE

Stormwater management

Kennedy Hall – Site A

*Rainwater tanks*. All roof areas will be used to harvest stormwater and be discharged into 4 No. existing, and 7 No. proposed above ground 3000 Lt tanks for reconnection to garden and internal reuse and onsite detention. The proportion of reuse to temporary onsite detention will be 50%/50%. The balance of the onsite detention will be existing 675 dia and proposed 600 dia underground stormwater pipes, together with the existing 4.92m3 underground rectangular stormwater tank located south centrally to existing Kennedy Hall. With the impervious coverage of the site at 95% the ability for the installation of open swales and/or rain gardens is non-existent due to the sheer lack of required open landscaped area. The ability to treat discharged stormwater onsite can only be done via an installed proprietary WSUD produce SPEL "Ecoceptor" or approved equivalent. The site will then discharge into the EXISTING legal point of discharge in Springvale Road via an orifice control pit.

#### Ann Street Carpark – Site B

*Rainwater pipes*. All bituminous pavement area will collect into a proposed 675 dia underground pipe for onsite detention only.

*Vegetated Sandy Loan Swale.* Water runoff from the bitumen car parks and driveways will fall towards the surrounding vegetation onsite and soak into the landscaped area. Though the site is being calculated at an impervious factor of 95% there is designated landscaped area set aside with room for open swale systems. The site will then discharge into the existing legal point of discharge in Ann Street via an orifice control pit.

#### WSUD ASSESSMENT RATING RESULTS

The measure of the WSUD best practise objectives will be via the use of the Music Model and Melbourne Water STORM calculator software.

#### What is eMUSIC ?

The Music Modelling tool software (eMUSIC) calculator is a software program developed as a method of the analysis of stormwater treatment methods using various source, treatment, and other node tools. The eMUSIC is used when the measurement of stormwater treatment varies from a simplified method to achieve Melbourne Water best practise Targets.

#### Best Practise Targets are:

Reduction in Total Suspended Solids.	80%
Reduction in Total Nitrogens.	45%
Reduction in Total Phosphorous	45%
Reduction in Litter from Typical Urban Design	70%

#### What is STORM ?

The Stormwater Treatment Objective Relative Measure (STORM) calculator is a tool that was developed by Melbourne Water as a method of simplifying the analysis of stormwater treatment methods. The STORM calculator is designed for the public to easily assess WSUD measures on any given property.

A STORM rating of 100% means that the objectives outlined above have been met. A site that does not incorporate any treatment measures will result in a STORM rating of 0%.

#### INFORMATION PROVIDED

#### General.

Functional Layout plan.

Kennedy Hall redevelopment.

- Summary OSD design report.
- Music Model Treatment Train.

#### Ann Street carpark.

- Summary OSD design report.
- Melbourne Water STORM report.

#### DRAINAGE STRATEGY

- Both drainage designs will be a gravity system.
- Both drainage designs will drain to the LPD via onsite detention and WSUD.

#### DETENTION DESIGN

- The runoff from the development was modelled in OSD4 to provide appropriate detention volume.
- The onsite detention will be a Q10 stormwater event.
- The site discharge will be a Q5 stormwater event.
- Coefficient of runoff for existing condition = 0.70 for Kennedy Hall and 0.74 for Ann Street Carpark
- Coefficient of runoff for finished development = 0.95 for both sites.
- The Tc to site is 5 min.
- The time travelled from discharge point to catchment outlet is as calculated.
- Site permissible site discharge is as calculated.

#### CONCLUSION

Kennedy Hall – Site A

- The required Q10 stormwater event volume is calculated at 53.78m3. The actual Q10 stormwater volume is calculated at 57.50m3
- The Music Treatment Train

### Best practise targets

tu Bets		
Reduction in Total Suspended Solids.	83.3%	80%
Reduction in Total Nitrogens.	50.9%	45%
Reduction in Total Phosphorous	74.8%	45%
Reduction in Litter from Typical Urban Design	100%	70%

Ann Street – Site B

- The required Q10 stormwater event volume is calculated at 12.87m3. The actual Q10 stormwater volume is calculated at 12.92m3
- The Melbourne Storm Rating is calculated at 219% greater than the required 100%.

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### **APPENDIX A**

KILLESTER COLLEGE - SENIOR CENTRE FUNCTIONAL LAYOUT PLAN.



### **APPENDIX B**

#### KENNEDY HALL ON-SITE STORMWATER DETENTION COMPUTATIONS

#### ON SITE STORMWATER DETENTION COMPUTATIONS

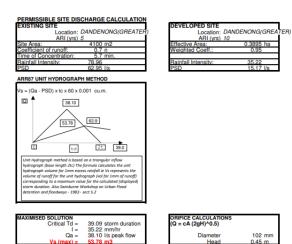
Project Name: Reference: Authority:

Date: Version No.

53.78 m3 @ Tc

Killesta SC Springvale Rd Springvale (a) 59 20 21 (Kennedy Hall Redevelopment) Greater Dandenong City Council 15/06/2021

0.3895 ha 0.95



REQUIRED STORAGE VOLUME =

Pipe Run	Dia.	Dia.	Length	Number of	Volume
om pit to pit)	(mm)	(actual)	(m)		(m3)
1 - EX	675.0	685.8	16.0	1	5.91
1-2	450.0	457.2	56.0	2	18.39
1-3	375.0	381.0	33.0	3	11.29
0	0.0	0.0	0.0	0	0.00
0	0.0	0.0	0.0	0	0.00
0	0.0	0.0	0.0	0	0.00
0	0.0	0.0	0.0	0	0.00
0	0.0	0.0	0.0	0	0.00
0	0.0	0.0	0.0	0	0.00
0	0.0	0.0	0.0	0	
	0.0	0.0	0.0		0.00
	0.0	0.0	0.0	Sub Total	0.00 35.60
	STORA	GE WITH	N PITS	Sub Total	35.60
AILABLE Pit No.	STORA(	GE WITHI W (m)	N PITS Design WL (ma	Sub Total	35.60 V (m3)
	STORA	GE WITH	N PITS	Sub Total	35.60
Pit No.	STORAC L (m) 0.60 0.60	<b>GE WITHI</b> W (m) 0.60 0.60	N PITS Design WL (ma 48.50 48.50	L 48.10 48.15	35.60 V (m3) 0.14 0.13
AILABLE S Pit No. 2 3 4	STORA0	<b>GE WITHI</b> W (m) 0.60 0.60 0.60	N PITS Design WL (ma 48.50	L 48.10 48.20	V (m3) 0.14 0.13 0.11
AILABLE S Pit No. 2 3 4	STORA0 L (m) 0.60 0.60 0.60	<b>GE WITHI</b> W (m) 0.60 0.60	N PITS Design WL (ma 48.50 48.50 48.50	L 48.10 48.15	35.60 V (m3) 0.14 0.13
AILABLE S Pit No. 2 3 4 u/g tank	STORA( L (m) 0.60 0.60 2.75	GE WITHI W (m) 0.60 0.60 0.60 2.75	N PITS Design WL (ma 48.50 48.50 48.50 48.50	L 48.10 48.15 48.20 48.30	35.60 V (m3) 0.14 0.13 0.11 4.92
AILABLE S Pit No. 2 3 4 u/g tank 0	STORAC L (m) 0.60 0.60 2.75 0.00	GE WITHI W (m) 0.60 0.60 0.60 2.75 0.00	N PITS Design WL (ma 48.50 48.50 48.50 48.50 48.50	Sub Total IL 48.10 48.15 48.20 48.30 0.00	35.60 V (m3) 0.14 0.13 0.11 4.92 0.00
AILABLE S Pit No. 2 3 4 ulg tank 0 0	STORAC L (m) 0.60 0.60 2.75 0.00 0.00	<b>SE WITHI</b> W (m) 0.60 0.60 0.60 2.75 0.00 0.00	N PITS Design WL (ma 48.50 48.50 48.50 48.50 48.50 48.50	IL           48.10           48.15           48.20           48.30           0.00           0.00	35.60 V (m3) 0.14 0.13 0.11 4.92 0.00 0.00
AILABLES Pit No. 2 3 4 4 4 0 0 0 0	STORA( L (m) 0.60 0.60 2.75 0.00 0.00 0.00	<b>3E WITHI</b> W (m) 0.60 0.60 0.60 2.75 0.00 0.00 0.00	N PITS Design WL (ma 48.50 48.50 48.50 48.50 48.50 48.50 48.50	Sub Total           IL           48.10           48.15           48.20           48.30           0.00           0.00           0.00	35.60 V (m3) 0.14 0.13 0.11 4.92 0.00 0.00 0.00 0.00
AILABLES Pit No. 2 3 4 4 0 0 0 0 0 0	STORA( L (m) 0.60 0.60 2.75 0.00 0.00 0.00 0.00	GE WITHI W (m) 0.60 0.60 0.60 2.75 0.00 0.00 0.00 0.00 0.00	N PITS Design WL (ma 48.50 48.50 48.50 48.50 48.50 48.50 48.50 48.50	Sub Total           IL           48.10           48.15           48.20           48.30           0.00           0.00           0.00           0.00           0.00	35.60 V (m3) 0.14 0.13 0.11 4.92 0.00 0.00 0.00 0.00 0.00

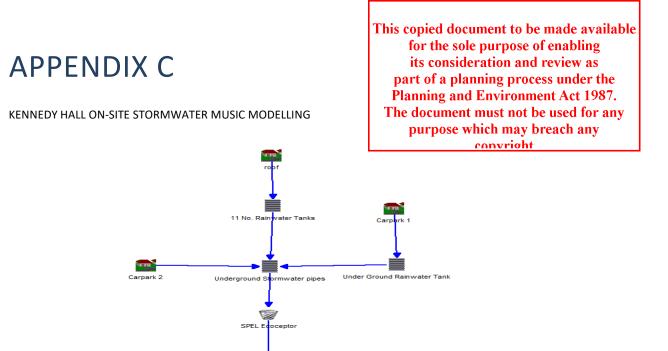
AILABLE STORAGE (OTHER) r tanks 11x3000 Lt

Sub To

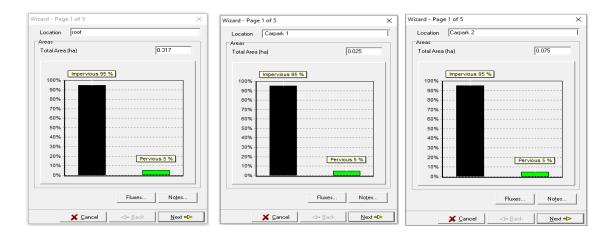
TOTAL STORAGE VOLUME = 57.50 m3

Page 1

102 mm



#### Music model treatment train



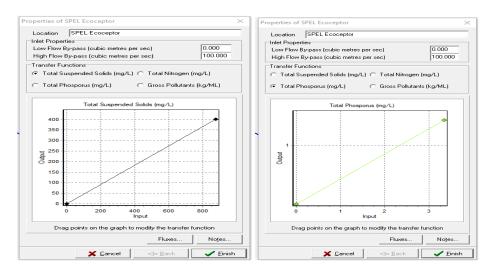
Receiving N

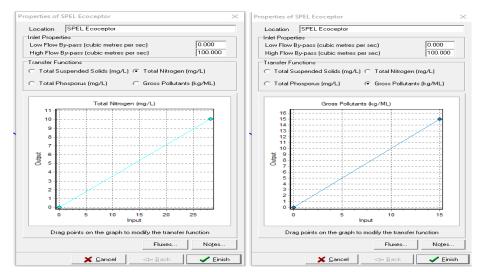
#### Source Node information

Properties of 11 No. Rainwater Tanks ×	Properties of Under Ground Rainwater Tank X	Properties of Undergrou
Location     11 No. Rainwater Tanks       Inlet Properties       Low Flow By-pass (cubic metres per sec)     0.0000       High Flow By-pass (cubic metres per sec)     100.000       Storage Properties     0.20       Volume below overflow (metres)     0.20       Surface Area (square metres)     25.0       Outlet Properties     0.verflow Pipe Diameter (mm)	Location         Under Ground Rainwater Tank           Intel Properties         0.0000           Low Flow By-pass (cubic metres per sec)         100.000           Storage Properties         Volume below overflow (netres)           Volume balow overflow (netres)         0.20           Surface Area (square metres)         5.0           Outlet Properties         0.20           Outlet Properties         5.0	Location Undergrou Intel Properties Low Flow By-pass (cub High Flow By-pass (cub Storage Properties Volume below overflow Depth above overflow Surface Area (square in Outlet Properties Overflow Pipe Diameter
Re-use Properties       If Use stored water for irrigation or other purpose       Annual Demand (kL/yr) scaled by daily PET       Daily Demand (kL/yr)       Daily Demand (kL/yr)       Monthly distribution of Annual Demand (kL/yr)       Fluxes       Notes	Re-use Properties       Use stored weter for ingesion or other purpose       Annual Demand (IL/yr) scaled by daily PET       Daily Demand (IL/day)       Monthly distribution of Annual Demand (IL/yr)       Fluxes       Notes	Re-use Properties Use stored water for Annual Demand (kL/yr Daily Demand (kL/day) Monthly distribution of A
X Cancel ⊲⇒ Back ✓ Finish	X <u>C</u> ancel <⊨ <u>B</u> ack	X Cance

Properties of Underground Stormwater pipes ×						
Location Underground Stormwater pipes	Ī					
Inlet Properties						
Low Flow By-pass (cubic metres per sec)	0.0000					
High Flow By-pass (cubic metres per sec)	100.000					
Storage Properties						
Volume below overflow pipe (kL)	36.08					
Depth above overflow (metres)	0.20					
Surface Area (square metres)	5.0					
Outlet Properties						
Overflow Pipe Diameter (mm)	50					
Re-use Properties						
Use stored water for irrigation or other purpose						
Annual Demand (kL/yr) scaled by daily PET	0.000					
Daily Demand (kL/day)	0.000					
Monthly distribution of Annual Demand (kL/yr)	0.000					
Fluxes Notes	More					
🗶 Cancel <> Back	🖌 <u>F</u> inish					

#### Rainwater Tanks node information.





SPEL Ecoceptor node Information

	Treatment Train Effectiveness - Receiving Node				×	
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purpose which may breach any convright						

Treatment Train Effectiveness Receiving node (Springvale Road.)

### APPENDIX D

#### ANN STREET CARPARK ON-SITE STORMWATER DETENTION COMPUTATIONS

Date: Version No.	15/06/2021 1	ty Council	•		
	AVAILABLE STOP	AGE WITH	IN PIPES		
		Dia		Number of	Volume
ANDENONG(GREATER)		(actual)	(m)		(m3)
	3 - 2 675.	685.8	33.0	1	12.19
0.10165 ha	0.00 0.0	0.0	0.0	0	0.00
0.95		0.0	0.0	0	0.00
	0 0.0	0.0	0.0	0	0.00
38.93	0 0.0	0.0	0.0	0	0.00
4.50 l/s		0.0	0.0	0	0.00
					0.00
					0.00
					0.00
	0 0.0	0.0	0.0		0.00
				Sub Total	12.19
					V (m3)
				40.00	0.38
					0.35
					0.00
					0.00
					0.00
					0.00
					0.00
					0.00
					0.00
					0.00
					0.00
	0.00	0.00	10.70		0.73
57 mm 0.53 m	AVAILABLE STOP	AGE (OTH	ER)	Sub Total	V (m3) 0.00 0.00 0.00 0.00
	0.95 38.93 4.50 Vs 57 mm	ANDENONG(GREATER) D 10165 ha 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.055 0.00 0.0	ANDENONG(GREATER) 0.10165 ha 0.35 0.35 0.36 0.0 0.450 0.0 0.00 0.00 0.00 0.0	ANDERNORG/GREATER) 0.01655 ha 0.45 0.45 0.45 0.45 0.45 0.50 hs 0.45 0.45 0.50 hs 0.50 hs 0	Pipe Run         Dia         Lendth         Number of (m)           0.10165 ha 0.055         0.05         685.8         33.0         1           0.055         0.00         0.0         0.0         0.0         0           38.93         0.0         0.0         0.0         0.0         0.0         0.0           0.00         0.0         0.0         0.0         0.0         0.0         0.0         0.0           0.00         0.0         0.0         0.0         0.0         0.0         0.0         0.0           0.00         0.0         0

ON SITE STORMWATER DETENTION COMPUTATIONS

Prepared by MCM 15/06/2021

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### **APPENDIX E**

ANN STREET CARPARK ON-SITE STORMWATER STORM MODELLING

# TransactionID: 1172160

Municipality:	GREATER DAND	ENONG				
Rainfall Station:	GREATER DAND					
Address:	Killesta Secondar	y Colleg				
	Springvale Road					
	Springvale					
	VIC					
Assessor:	Martin Masina					
Development Type:	Commercial/Retai	1				
Allotment Site (m2):	1,070.00					
STORM Rating %:	219					
Description	Impervious Area (m2)	Treatment Type	Treatment Area/Volume (m2 or L)	Occupants / Number Of Bedrooms	Treatment %	Tank Water Supply Reliability (%)
Carpark 1	510.00	Infiltration Sandy Loam	30.00	0	215.70	0.00
Carpark 2	510.00	Infiltration Sand	30.00	0	222.00	0.00

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Date Generated:

15-Jun-2021

Program Version: 1.0.0

Note the Ann Street carpark has been divided into two half's (Carpark 1 and Carpark 2) so that the full area of landscaping can be maximised.