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Traffic Impact Assessment Report

Westbourne Grammar School - Year 5-8 Learning Centre 300-340 Sayers Road, Truganina





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Westbourne Grammar School May 2023



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Front Cover Image: Year 5-8 Learning Centre Main Plaza 3D Visualisation, May 2023, Baldasso Cortese



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1 INTRODUCTION

1.1 Background

Transport & Traffic Solutions Pty Ltd (T&TS) has been engaged by Westbourne Grammar School to prepare a Traffic Impact Assessment Report in support of their planning permit application to construct a Year 5-8 Learning Centre at their Truganina Campus, located at 300 – 340 Sayers Road, Truganina. An existing planning permit, permit no. WYP0543.08 applies to the site.

The construction of the new building will require the following car park changes:

- Closure of the internal north-south aligned circulation road including the removal of 18 staff car parking spaces located to the west of the Holy Trinity Hall, and
- Modification of the east-west aligned circulation road located north of Sayers Road to include a turn area at its western end (south-west of Holy Trinity Hall).

Construction of the new Year 5-8 Learning Centre is expected to commence early December 2023 with completion scheduled for late June 2025. Building occupation will occur July 2025. At this time, enrolment & staff numbers are expected to increase by 182 students and 36 staff, respectively.

We understand that the school benefits from Existing Use Rights and that the school is not subject to planning permit conditions in relation to staff or student numbers, or car parking.

The aim of this report is to determine what impact these changes will have on the operation of the school site when the Year 5-8 Learning Centre opens in July 2025.

1.2 References

The following references were used to assist in the preparation of this report:

- Department of Transport and Planning, VicPlan and Planning Schemes Online, State Government of Victoria, Accessed May 2023;
- Austroads Guide to Road Design, Austroads Guide to Traffic Management, Australian Standards, and the VicRoads Supplement to the Austroads Guide and Australian Standards, as detailed in this report;
- 5-8 Learning Centre, Westbourne Grammar School, Detailed Site Plan, Baldasso Cortese, Drawing No A-TP-102, Rev B, 24 May 2023; and
- Other documents as referenced within this report.





2 EXISTING CONDITIONS ASSESSMENT

2.1 Site Location & Details

Westbourne Grammar School is located at 300-340 Sayers Road, Truganina. It is bounded by the Forsyth Road Drain, vacant land and existing residential dwellings to the north, Marquands Road to the east, Sayers Road to the south, and existing residential dwellings to the west. The total site area is approximately 36.4 hectares. Refer Figure 2.1 for the school location.

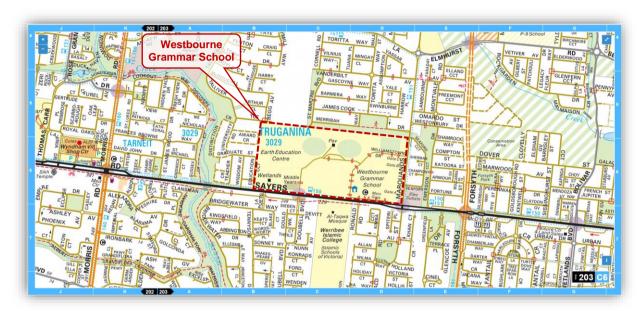


Figure 2.1: Locality Plan¹

Westbourne Grammar School offers early years, primary and secondary school learning from multiple buildings located across the site as follows:

- South-East Between Williamstown Drive, Marquands Road and Sayers Road for Years 3 to 8 and Years 10 to 12 students, and
- South-West Either side of Doubell Boulevard for ELC (Amici), Prep to Year 2 (Winjeel), and Year 9 Students (Geoffrey Ryan Centre).

The school also has the various sporting facilities located across the site as follows:

- Sports and aquatic centre & gymnasium located to the south of Williamstown Drive, west of Marquands Road.
- Senior sports ovals (Williams and College Ovals) including a sports pavilion to the north of Williamstown Drive and Doubell Boulevard, and
- Junior sports ovals to the west of Williamstown Drive north of Sayers Road.

Other infrastructure located within the site include internal access roads, car parks and bus parking.

2.2 Access & Off-street Car Parking

Refer Figure 2.2 for a site plan of the school site detailing the location of the school access points and off-street car parks.



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2.2.1 Site Access

Access to the site is provided from the existing road network as follows:

- > Sayers Road via the:
 - Signalised intersection of Sayers Road and Doubell Boulevard, allowing all movements (Doubell Gate, 340 Sayers Road).
 - Unsignalised one-way entry point located 190 metres west of Marquands Road, (Main Gate, 300 Sayers Road), allowing left in and right in movements.
- Marguands Road via:
 - Gate A Unsignalised one-way exit point located 85 metres north of Sayers Road, allowing left out and right out movements.
 - Gate B Emergency Vehicle Access point located 130 metres north of Sayers Road (locked).
 - Sports & Aquatic Deliveries & Trade Access gate located 90 metres north of Seaford Circuit (locked).
 - Gate C Unsignalised intersection of Marquands Road and Williamstown Drive located 100 metres south of Seaford Circuit, allowing all movements.
 - Gate D Unsignalised one-way exit point located opposite Seaford Circuit, allowing left out movement only.

Pedestrian access to the site is provided from:

- Doubell Boulevard West side of the signalised intersection.
- Marquands Road North-west of the Sayers Road/ Marquands Road intersection.
- Marguands Road North of Gate C.

2.2.2 Off-street Car Parking

A total of 621 off-street car parking spaces are provided within the site. Details as follows.

- > Visitor Parking A total of 262 off-street visitor car parking spaces are provided as follows:
 - Zone A, Williamstown Drive 139 No. 90 degree angle car parking spaces, 2 No. accessible car parking space for disabled users, 9 No. drop off/ pick up car parking spaces, and 1 No. emergency vehicle space.
 - Zone B, Amici & Winjeel 24 No. 45 degree angle car parking spaces and 2 No. accessible car parking spaces for disabled users. All car parking spaces are controlled with a 10 minute time limit.
 - Zone C, Doubell Boulevard 29 No. 45 degree angle car parking spaces and 12 No. drop off/ pick up car parking spaces.
 - Zone D & E, Sports Pavilion 45 No. 90 degree car parking spaces.
- Staff Parking A total of 330 off-street staff car parking spaces are provided as follows:
 - o Zone F, Marquands Road 27 No. 45 degree angle car parking spaces.
 - Zone G & H, Sayers Road 86 No. 45 degree angle car parking spaces and 3 No. accessible car parking spaces for disabled users.
 - o Zone I to N, Central 159 No. 45 & 90 degree angle car parking spaces.
 - Zone O, The Geoffrey Ryan Centre 44 No. 90 degree angle car parking spaces and
 1 No. accessible car parking spaces for disabled users.
 - Zone Q, Marquands Road 10 No. 90 degree angle car parking spaces.

This copied document Maintenance evailable by Vehicle Parking – A total of 29 off-street maintenance and delivery for the sole purposahidle publing spaces are provided as follows:

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- Zone G & H, Delivery Vehicles 4 No. Parallel parking spaces (courier van or similar).
- Zone P, Maintenance & Delivery Vehicles 15 No. 90 degree angle parking spaces, 1
 No. accessible car parking space for disabled users, and 1 No mini-bus parking.
- Zone Q, Sports & Aquatic Delivery & Trade Vehicles 9 No. 90 degree angle car parking spaces.

Bus parking (Zone R) with capacity for 17 buses is provided within the site to the south-east of the college oval. Access to the bus zone is provided via the Gate C access point and Williamstown Drive. 1 No. mini-bus parking space is also provided within Zone P.

Waste collection occurs from two waste collection areas, one located south of the Amici Centre and the other located south-east of the College Oval.

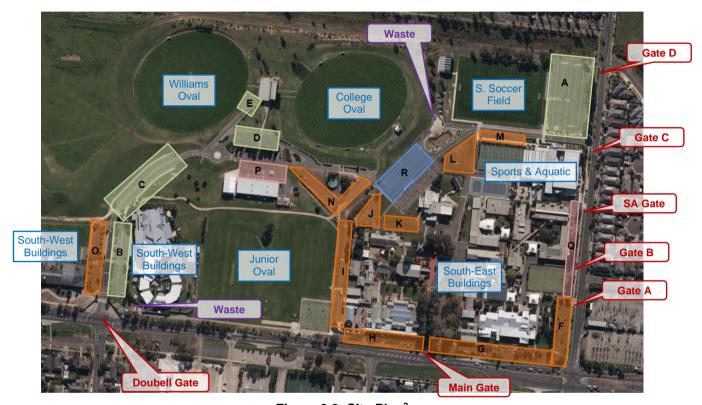


Figure 2.2: Site Plan²

2.3 Road Network

2.3.1 Road Network Characteristics

An inspection of the road network adjacent to the school was undertaken on Tuesday 14 June 2022, between the hours of 8:00am to 10:00am and 2:30pm to 4:00pm. Details of the site inspection are as follows.

2.3.1.1 Sayers Road (B77)

Sayers Road (B77) is a sealed two-lane two-way declared arterial road managed by the Department of Transport. It runs in an east-west direction between Old Geelong Road in the east and the Regional Rail Link in the west where it ends.

Adjacent to the school, the cross-sectional profile of Sayers Road varies to include turn lanes at its intersections with Ronald Road, Doubell Boulevard (signalised intersection), the school access point and the Al-Taqwa College access point; and a divided cross-sectional profile at its intersection with Marquands Road (signalised intersection). Refer Figure 2.3 and Figure 2.4

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² https://www.bing.com/maps/aerial, accessed May 2023.



A posted speed limit of 60 km/h applies to Sayers Road. A time-controlled school zone speed limit of 40 km/h applies to Sayers Road (8-9:30am and 2:30-4pm school days) between Marquands Road and Doubell Boulevard.

Referring to the Data VIC website³, Sayers Road adjacent to the school site carried approximately 13,000 vehicles per day in 2018.



Figure 2.3: Sayers Rd, looking east to Doubell Blvd



Figure 2.4: Sayers Rd, looking west from the 300 Sayers Rd Access Point

2.3.1.2 Marquands Road

Marquands Road is a sealed two-lane two-way undivided road which runs in a north-south direction between Leakes Road in the north and Sayers Road and Alison Street in the south. Referring to Wyndham City Register of Public Roads, Marquands Road is classified as a "Local Access" road.

Adjacent to the school, Marquands Road comprises a 6 metre wide carriageway with a 2.6 metre wide indented parking lane on the east side of the carriageway. A 1.5 metre and 2.5 metre wide footpath and varying width grass verge is provided on the east and west sides of Marquands Road, respectively. Refer Figure 2.5 and Figure 2.6.

A posted speed limit of 60 km/h applies to Marquands Road. A time-controlled school zone speed limit of 40 km/h applies to Marquands Road (8-9:30am and 2:30-4pm school days) between James Cook Road and Sayers Road.



Figure 2.5: Marquands Road, looking north from Gate C



Figure 2.6: Marquands Road, looking south from Gate C

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https://vicroadsmaps.maps.arcgis.com/apps/mapviewer/index.html?layers=3a4a2e376f16

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2.3.2 Existing Site Access Volumes

Traffic volume surveys were completed by Nationwide Traffic Surveys Pty Ltd at the Doubell Gate (signalised intersection of Sayers Road/ Doubell Boulevard), Sayers Road Main Gate (one-way entry point), the Marquands Road Gate A (one-way exit point), the Gate C (two-way access point), and Gate D (one-way left out only exit point) on Friday 20 May 2022 between 8:00am-9:30am and 2:30pm-4:30pm.

Refer Figure 2.7 for the existing intersection turning movement volumes at these intersections during the 30 minute AM & PM Peak periods recorded between 8:15am to 8:45am and 3:30pm to 4:00pm respectively. Note, the recorded traffic volumes include staff vehicles entering (AM Peak) and exiting (PM Peak) the site during the AM & PM Peak periods. Further, visitors wanting to access the Zone A car park are required to enter the site at Gate C and exit the site at Gate D. Only staff and buses are allowed to exit the site from Gate C.

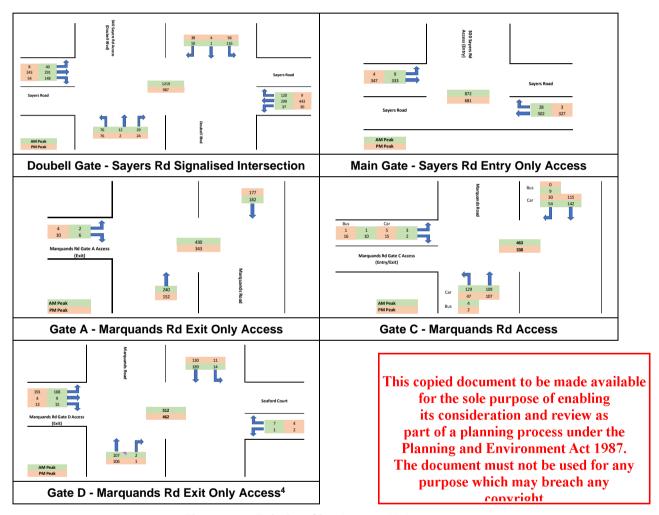


Figure 2.7: Existing Site Access Volumes

Based on the intersection turning movement volumes at all access points, a total of 392 (51%) vehicles and 372 (49%) vehicles (excluding buses) enter and exit the college respectively during the 30 minute AM Peak period, and a total of 103 (23%) vehicles and 340 (77%) vehicles enter and exit the college respectively during the 30 minute PM Peak period. A breakdown of the number of vehicles entering and exiting each access point in percentage format (traffic distribution) is provided in Table 2.1.



⁴ Not all vehicles existing Gate D turned left out of the site as required.



Table 2.1: AM & PM Peak Traffic Volume Distribution (%)

	AM Peak		PM I	Peak
Access Point	In	Out	In	Out
Doubell Gate	44%	45%	18%	29%
Main Gate	9%	-	7%	-
Gate A	-	2%	-	4%
Gate C	47%	1%	75%	6%
Gate D	-	52%	-	61%
Total	100%	100%	100%	100%

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2.3.2.1 Existing Trip Generation Rate

Based on a total of 1,370 students being present at Westbourne Grammar School during the time of the survey, it can be concluded that the school generates approximately 0.56 two-way vehicle trips per student during the 30 minute AM Peak period and 0.32 two-way vehicle trips per student during the 30 minute PM peak period. These trips include staff vehicles entering (AM) and exiting (PM) the school during the peak times.

2.3.2.2 Sayers Road & Marquands Road Two-way Traffic Volume

The two-way AM and PM Peak hour traffic volume recorded on Sayers Road west of Doubell Boulevard and on Marquands Road north of Seaford Circuit is:

- Sayers Road AM Peak = 1,557 vph, PM Peak = 1,780 vph
- ➤ Marguands Road AM Peak = 710 vph, PM Peak = 695 vph

Based on industry standard that 10% of the daily traffic volume occurs within the peak hour, it is estimated that Sayers Road west of Doubell Boulevard has a daily traffic volume equivalent to 17,800 vehicles per day, where-as Marquands Road north of Seaford Circuit has a daily traffic volume equivalent to 7,100 vehicles per day. This volume of traffic is just below the expected capacity of a two lane two-way arterial road (maximum 18,000 vehicles per day) and over the capacity of a local access road (maximum 3,000 vehicles per day). With a no-stopping zone implemented on the west side of Marquands Road and the provision of indented parking on the east side, it is expected that Marquands Road can carry up to 7,000 vehicles per day, like a connector street standard.

2.3.3 Site Access Operational Performance

A check of the operational performance of the existing site access points was undertaken using SIDRA Intersection 9.1 software to understand the current operational performance of the site access points. A summary of the Degree of Saturation, Average Delay, Level of Service, and 95% Back of Queue Distance results of the analysis for the "total approach" of the intersections with the existing traffic volumes are provided in Table 2.3 and Table 2.4. Full results are provided in Appendix A.

In accordance with the VicRoads supplement to AGTM, Part 3: Traffic Studies and Analysis (2013), when evaluating intersection performance during capacity analysis and design, the target maximum degree of saturation of the critical (maximum) movement is 0.9 (desirable) and 0.95 (maximum) at a signalized intersection, and 0.8 (desirable) and 0.85 (maximum) at an unsignalized intersection, including roundabouts.





Table 2.2: SIDRA Summary Results - Doubell Gate, Sayers Rd Signalised Intersection

Approach Leg	Doubell Blvd (South)	Sayers Road (East)	Doubell Blvd (North)	Sayers Road (West)	Total Intersection	
AM Peak						
Degree of Saturation	0.510	0.841	0.580	0.837	0.841	
Average Delay (sec)	65.5	64.0	50.4	64.3	62.4	
Level of Service	Е	Е	D	E	E	
95% Queue Distance (m)	64.7	242.9	73.2	243.8	243.8	
PM Peak						
Degree of Saturation	0.441	0.872	0.437	0.529	0.872	
Average Delay (sec)	59.6	47.7	44.3	27.0	42.2	
Level of Service	Е	D	D	С	D	
95% Queue Distance (m)	56.4	340.0	42.7	117.9	340.0	

The Doubell Gate, Sayers Road Signalised Intersection is operating within its limits with a Level of Service E, Degree of Saturation below 0.85, Average Delay less than 65 seconds, and Queues less than 250 metres long during the AM Peak period, and a Level of Service D, Degree of Saturation below 0.88, Average Delay less than 45 seconds, and Queues 340 metres long during the PM Peak period.

Table 2.3: SIDRA Summary Results - Main Gate, Sayers Rd Entry Only Access

Approach Leg	Sayers Road (East)	Sayers Road (West)	Total Intersection
AM Peak			
Degree of Saturation	0.520	0.347	0.520
Average Delay (sec)	0.6	0.2	0.5
Level of Service	-	-	-
95% Queue Distance (m)	2.3	0	2.3
PM Peak			
Degree of Saturation	0.336	0.355	0.355
Average Delay (sec)	0.1	0.2	0.1
Level of Service	-	-	-
95% Queue Distance (m)	0.2	0	0.2



The Main Gate, Sayers Road Entry Only Access Intersection is operating below its capacity with a Degree of Saturation below 0.60, Average Delay less than 1 second, and Queues less than 1 metre long during both the AM Peak and PM Peak periods.

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Table 2.4: SIDRA Summary Results - Gate A, Marquands Rd Exit Only Access

Approach Leg	Marquands Road (South)	Marquands Road (North)	School Access Road (West)	Total Intersection
AM Peak				
Degree of Saturation	0.248	0.193	0.023	0.248
Average Delay (sec)	0.1	0.1	4.7	0.1
Level of Service	-	-	А	-
95% Queue Distance (m)	0	0	0.5	0.5
PM Peak				
Degree of Saturation	0.159	0.190	0.030	0.190
Average Delay (sec)	0	0.1	3.2	0.2
Level of Service	-	-	А	-
95% Queue Distance (m)	0	0	0.7	0.7

The Gate A, Marquands Road Exit Only Access Intersection is operating well below its capacity with a Degree of Saturation below 0.30, Average Delay less than 1 second, and Queues less than 1 metre long during both the AM Peak and PM Peak periods.

Table 2.5: SIDRA Summary Results - Gate C, Marquands Rd Access

Approach Leg	Marquands Road (South)	Marquands Road (North)	Williamstown Drive (West)	Total Intersection
AM Peak				
Degree of Saturation	0.312	0.293	0.095	0.312
Average Delay (sec)	3.3	4.1	9.3	3.9
Level of Service	-	-	А	-
95% Queue Distance (m)	12.3	10.5	3.3	12.3
PM Peak ⁵				
Degree of Saturation	0.993	1.140	0.135	1.140
Average Delay (sec)	10.4	69.0	5.0	34.9
Level of Service	-	-	А	-
95% Queue Distance (m)	101.5	102.3	4.3	102.3

The Gate C, Marquands Road Access Intersection is operating well below its capacity during the AM Peak period with a Degree of Saturation below 0.32, Average Delay less than 4 seconds, and Queues less than 13 metres long. Where-as during the PM Peak period, the Gate C, Marquands Road Access Intersection is operating above acceptable limits and its capacity with a Degree of Saturation less than 1.2, Average Delay of around 35 seconds, and Queues up to 103 metres long.

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A capacity adjustment factor was applied to the Marquands Road approach lanes to achieve a similar queue length to and review as observed conditions, approximately 100m (14 Cars) on the southern and northern approach.

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It was observed during the site visit that the performance results of the intersection are associated with downstream blockages created by school children crossing the Zone A car park access point. Therefore, it is recommended that the school's own children crossing supervisor stop children from crossing the car park access point once they see the Williamstown Drive vehicle queues extending onto Marquands Road.

Table 2.6: SIDRA Summary Results - Gate D, Marquands Rd Exit Only Access

Approach Leg	Marquands Road (South)	Seaford Circuit (East)	Marquands Road (North)	School Access Road (West)	Total Intersection
AM Peak					
Degree of Saturation	0.116	0.041	0.214	0.347	0.347
Average Delay (sec)	0.2	11.8	0.4	1.9	1.1
Level of Service	-	В	-	А	-
95% Queue Distance (m)	0.3	0.9	0	11.9	11.9
PM Peak					
Degree of Saturation	0.114	0.023	0.144	0.349	0.349
Average Delay (sec)	0.1	9.3	0.5	1.5	1.0
Level of Service	-	А	-	А	-
95% Queue Distance (m)	0.1	0.5	0	11.9	11.9

The Gate D, Marquands Road Exit Only Access Intersection is operating well below its capacity with a Degree of Saturation below 0.40, Average Delay less than 2 seconds, and Queues less than 12 metres long during both the AM Peak and PM Peak periods.

In summary, all site access points are operating within acceptable limits during school drop off and pick up periods, apart from the Gate C, Marquands Road Access Intersection during the PM Peak period.

2.4 Car Parking

2.4.1 Car Parking Occupancy Survey

A Car Parking Occupancy Survey was completed of the existing off-street car parking facilities within the school site and the on-street car parking facilities located adjacent to the school site by Nationwide Traffic Surveys on Friday 20 May 2022 between 8:00am-10:00am and 2:00pm-4:00pm at 15 minute intervals. Refer Figure 2.8 for the surveyed locations.

The car parking occupancy survey was completed to determine the existing off-street and on-street car parking demand currently being generated by the school.

The school site has a total car parking supply equivalent to 621 car parking spaces which is made up of 359 spaces for staff, maintenance, and trade vehicle use and 262 spaces for visitor use. Refer Appendix B for the car parking occupancy survey results.





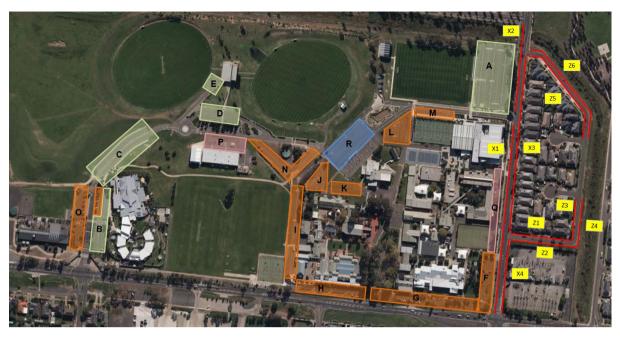


Figure 2.8: Car Parking Occupancy Survey Map

2.4.1.1 Off-street Visitor Car Parking Survey Results

The Zone A (Williamstown Drive) car park consists of 139 No. 90 degree angle car parking spaces, 2 No. accessible car parking space for disabled users, 9 No. drop off/ pick up car parking spaces, and 1 No. emergency vehicle space.

The car parking occupancy survey results for the Zone A car park are summarised in Figure 2.9.



Figure 2.9: Car Parking Occupancy Survey Results - Zone A Car Park

As illustrated in Figure 2.9, during the AM period, the number of occupied car parking spaces increases gradually from 3 (2%) spaces recorded at 8:00am to a peak of 22 (15%) spaces recorded at 8:30am. After this time, the number of car parking spaces occupied falls rapidly to 12 (8%) spaces recorded at 8:45am and then 2 (1%) spaces recorded after 9:15am.

During the PM period, the number of occupied car parking spaces increases from 8 (5%) spaces recorded at 2:00pm to a peak of 1486 (99%) spaces recorded at 3:30pm. After this time, the number of car parking spaces occupied falls rapidly to 42 (28%) spaces recorded at 3:45pm and then 24 (16%) spaces recorded at 4:00pm.

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its consid<mark>eration and review as The 2 No. accessible car parking spaces were empty.</mark>

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The Zone B (Amici & Winjeel), Zone C (Doubell Blvd), and Zone D & E (Sports Pavilion) car parks consists of the following number of car parking spaces:

- Zone B 24 No. 45 degree angle car parking spaces and 2 No. accessible car parking spaces for disabled users. All car parking spaces are controlled with a 10 minute time limit.
- Zone C 29 No. 45 degree angle car parking spaces and 12 No. drop off/ pick up car parking spaces.
- Zone D & E 45 No. 90 degree car parking spaces.

The car parking occupancy survey results for the Zone B to E car park are summarised in Figure 2.10.

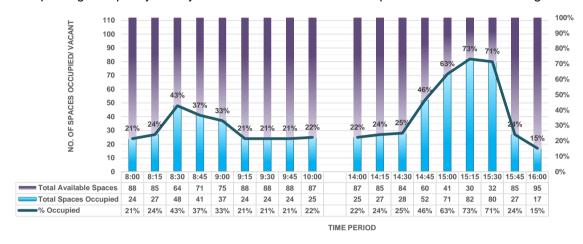


Figure 2.10: Car Parking Occupancy Survey Results - Zone B to E Car Park

As illustrated in Figure 2.10, during the AM period, the number of occupied car parking spaces increases gradually from 24 (21%) spaces recorded at 8:00am to a peak of 48 (43%) spaces recorded at 8:30am. After this time, the number of car parking spaces occupied falls to 41 (37%) spaces recorded at 8:45am and then 24 (11%) spaces recorded at 9:15am.

During the PM period, the number of occupied car parking spaces increases from 25 (22%) spaces recorded at 2:00pm to a peak of 82 (73%) spaces recorded at 3:15pm. After this time, the number of car parking spaces occupied falls to 80 (71%) spaces recorded at 3:30pm and then rapidly to 27 (24%) spaces recorded at 3:45pm.

As illustrated in Figure 2.10, the demand for car parking within the Zone B to E car parks is spread out over a half hour period which is a result of the staggered school end times for Prep (3:05pm), Grade 1 (3:15pm), Grade 2 (3:25pm) and Year 9 (3:30pm) levels. The staggered end times contribute to these car parks not reaching capacity. Where-as with the Zone A car park, a surge in demand for car parking is experienced at 3:30pm, with the car park arriving close to its capacity, refer Figure 2.9. This is a result of the school end times for the middle and senior schools occurring at the same time.

Based on the car parking survey results as illustrated in Figure 2.9 and Figure 2.10, the school site generates a peak off-street visitor car parking demand equivalent to 228 car parking spaces recorded at 3:30pm. Currently, the school has an oversupply of visitor car parking spaces equivalent to 34 car parking spaces⁷. This oversupply in car parking is located within the Zone D & E car parks.

2.4.1.2 Off-street Staff Car Parking Survey Results

Zone F (Marquands Road), Zone G & H (Sayers Road), Zone I to N (Central), Zone O (The Geoffrey Ryan Centre & part Amici & Winjeel), Zone P (Maintenance & Deliveries), and Zone Q (Sports & Aquatic) consists of 326 No. angled car parking spaces, 4 No. accessible car parking space for disabled users, and 29 No. maintenance and trade vehicle spaces.

The car parking occupancy survey results for the Zone F to Q car parks are summarised in Figure 2.11.

⁷ A total of 35 car parking spaces were available within the Zone D & E car parks. The reason why the survey results show 34 review as available car parking spaces is that 1 vehicle was double parked within the Zone C car park.

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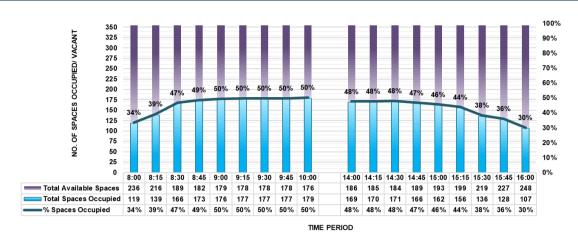


Figure 2.11: Car Parking Occupancy Survey Results - Zone F to Q Car Park

As illustrated in Figure 2.11, during the AM period, the number of occupied car parking spaces increases gradually from 119 (33%) spaces recorded at 8:00am to a peak of 179 (50%) spaces recorded at 10:00am.

During the PM period, the number of occupied car parking spaces averages 169 (47%) spaces recorded between 2:00pm to 3:00pm. After this time, the number of occupied car parking spaces reduces gradually to 107 (30%) spaces recorded at 4:00pm.

As illustrated in Figure 2.11, the school has an oversupply of staff car parking spaces equivalent to 176 car parking spaces.

2.4.1.3 **On-street Visitor Car Parking Survey Results**

Marquands Road between James Cook Drive and Sayers Road has a total capacity for up to 41 onstreet car parking spaces (13 east side & 28 west side), where-as Seaford Circuit has a total capacity for up to 64 on-street car parking spaces (28 north side & 36 south side). The car parking occupancy survey results are summarised in Figure 2.12.

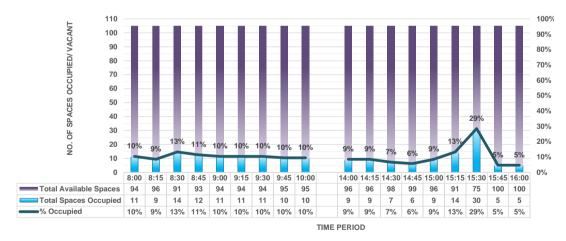


Figure 2.12: On-street Car Parking Occupancy Survey Results

As illustrated in Figure 2.12, during the AM period, the number of occupied car parking spaces increases gradually from 11 (10%) spaces recorded at 8:00am to a peak of 14 (13%) spaces recorded at 8:30am.

During the PM period, the number of occupied car parking spaces increases from 9 (9%) spaces recorded between 2:00pm & 3:00pm to a peak of 30 (29%) spaces recorded at 3:30pm. After this time, the number of car parking spaces occupied falls rapidly to 5 (5%) spaces recorded at 3:45pm and 4:00pm.

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Traffic Impact Assessment Report

Based on the results of the on-street car parking occupancy survey, during the AM period it is estimated a total of five (5)8 school users parked on street, where-as during the PM peak period a total of twenty five (25)9 school users parked on street.

Existing Car Parking Supply and Demand Summary 2.4.1.4

Referring to Section 2.4.1.1, the school site has car parking supply for visitor use equivalent to 262 spaces and generates a peak visitor car parking demand equivalent to 22810 car parking spaces at 3:30pm. Based on a total of 1,370 students¹¹ (85%) being present on site during the time of the survey, Westbourne Grammar School generates a demand for visitor off-street car parking equivalent to 0.17 car parking spaces per student on the site at any time.

Referring to Section 2.4.1.2, the school site has car parking supply for staff, maintenance and trade vehicle use equivalent to 355 spaces and generates a peak staff car parking demand equivalent to 180 car parking spaces at 10:00am. Based on a total of 345 staff¹² (93%) being present on site during the time of the survey, Westbourne Grammar School generates a demand for visitor off-street car parking equivalent to 0.52 car parking spaces per staff member present on the site at any time.

Transport, Bicycle Facilities & Path Network

2.5.1 **Public Transport**

Bus Route 150 Williams Landing Station - Tarneit Station operates along Sayers Road to the south of the school. The closest bus stops that service both route directions are located directly south of the school. Buses service these stops every 15-17 minutes prior to the school start and after the school end times.

Bus Route 151 Williams Landing Station - Tarneit Station via Westmeadows Lane operates along Marquands Road to the east of the school. The closest bus stops that service both route directions are located directly east of the school, south of Gate C. Buses service these stops every 18 minutes prior to the school start and after the school end times.

2.5.2 Private Transport

The college currently operates seventeen (17) bus services prior to the school start and after the school end times and six (6) late buses for after school activities. Details of the bus service are as follows:

Before & After School -

Bus Route 2 - Werribee South, Bus Route 5 - Wyndham Vale, Bus Route 7 - St Albans / Sunshine / Keilor Downs / Cairnlea, Bus Route 8 - Williamstown, Bus Route 10 - Footscray / Newport, Bus Route 11 - Spotswood / Altona, Bus Route 12 - Yarraville / Footscray / Newport / Altona North, Bus Route 13 - Spotswood / Newport / Williamstown, Bus Route 15 - Altona / Altona Meadows / Laverton, Bus Route 16 - Williamstown, Bus Route 17 - Yarraville / Spotswood / Williamstown, Bus Route 18 - Rifle Range Estate / Williamstown, Bus Route 19 -Ascot Vale / Footscray / West Footscray, Bus Route 20 - Hillside / Sydenham / Caroline Springs, Bus Route 21 - Sanctuary Lakes, and Bus Route 22 - Point Cook/Boardwalk, Williamstown Campus, and Williamstown Campus - Truganina Campus to Williamstown Campus.

Late Bus -

Bus Route 1 - Williamstown, Spotswood, Yarraville, Seddon, Kingsville, Bus Route 2 - Hoppers Crossing, Laverton, Sanctuary Lakes, Point Cook Rd, Altona Meadows, Bus Route 3 -Williamstown, Williamstown North, Bus Route 4 - Hoppers Crossing, Werribee Plaza, Werribee, Wyndham Vale, Bus Route 5 - Sunshine, Footscray, Avondale Heights, Keilor East, Deer Park, Caroline Springs, and Bus Route 6 - Altona Nth. Newport.

Approximately 556 (37%) children use the bus service to arrive to school in the morning and 637 (42%) students use the bus service to return home in the evening.

¹² A total of 372 staff are employed by the school in 2022.

8 Difference between the 14 car parking spaces occupied at 8:30am and the 9 car parking spaces occupied at 8:15am.
9 Difference between the 30 car parking spaces occupied at 3:30pm and the 5 car parking spaces occupied at 8:15am.

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¹⁰ Excludes the 25 car parking spaces occupied on-street.

^{11 2022} Primary, Middle, and Senior School Enrolments = 1,520 students, 2022 ELC Enrolments = 98 Childrenderation and review as



2.5.3 Bicycle Parking

A total of 48 bicycle parking spaces are provided within the school site as follows:

- ➤ Horsburgh Centre 4 spaces via 2 hoops.
- ➤ The Geoffrey Ryan Centre 24 spaces via 12 hoops.
- ➤ Sports & Aquatic Centre 10 spaces via 5 hoops.
- ➤ Senior School Building / Gate B 10 spaces at a bicycle rack.

2.5.4 Path Network

The site is surrounded by limited off-road bicycle facilities. A shared path is provided on the north side of Sayers Road between 352 Sayers Road and Skeleton Waterholes Creek, south side of Sayers Road between Forsyth Road and Skeleton Waterholes Creek, and within the Skeleton Waterholes Creek reserve.

A 2.0 metre wide footpath is provided on the north side of Sayers Road between:

- Marquands Road and the eastbound bus stop (located approximately 115 metres west of Marquands Road).
- The Doubell Boulevard school pedestrian entry point and the eastbound bus stop (located approximately 100 metres west of Doubell Boulevard).

A 1.5 metre and 2.5 metre wide footpath is provided on the east and west side of Marquands Road, respectively.

2.6 Background Transport Review

Discussions with both Wyndham City Council and the Department of Transport & Planning on the future upgrade of Sayers Road were held via email correspondence back in June 2022. Details of the email correspondence are provided below:

- > Wyndham City Council
 - o There are no plans to upgrade Sayers Road soon.
 - It is assumed Sayers Road is to be upgraded to a four lane divided carriageway as this
 is what was approved at the Marquands Road intersection.
 - Sayers Road has been nominated as a C2 Strategic Cycling Corridor (all ages and abilities). To accommodate all ages and abilities, Sayers Road will require an off-road bike path or separated bike lanes.

Department of Transport

- The future configuration of Sayers Road adjacent to the school has not yet been determined, noting that its ultimate form (4 lanes v 6 lanes) is subject to ongoing investigations by Major Road Projects Victoria (section between Derrimut Road and Palmers Road).
- Part of the road may be delivered as 6 lanes in some sections including where the school is located. Given that Leakes Road to the north will be delivered as a 6 lane road, it is unlikely that the whole of Sayers Road would also be delivered as such, given their convergence at the freeway intersection.





3 PROPOSED DEVELOPMENT

The proposal consists of constructing a new school building for years 5 to 8 (Year 5-8 Learning Centre). The building will be located to the north-east of the Junior Oval and require the following changes to the Zone H, I & J car parks:

- Zone H (Staff Car Park)
 - Addition of a right turn arrow to the existing left turn only arrow located north of the main gate.
 - Conversion of the one-way westbound circulation road (west of the main gate) to a twoway circulation road.
 - Addition of a turn area at the western end of the circulation road to allow vehicles parked in the 45 degree angled car parking spaces to turn around and exit the site at Marquands Road Gate A exit point. The turn area will also be used by delivery vehicles up to 8.8 metres long, so that they can access the loading bays located on the north side of the Zone H car park.
- Zone I (Staff Car Park)
 - o Closure of the internal north-south aligned circulation road.
 - Removal of 18 staff car parking spaces.
 - o Reinstatement of the area with pedestrian paths and landscaping.
- Zone J (Staff Car Park)
 - o Removal of pavement arrows so that staff vehicles can travel in any direction.

Refer Appendix C for the proposed Year 5-8 Learning Centre detailed site plan, and Appendix D for swept path assessment drawing no. 23011WG-00-101, illustrating an 8.8 metre long Medium Rigid Vehicle performing a three-point turn at the Zone H turn area and a B99 and B85 vehicle passing each other simultaneously at the Main Gate access point/ Zone H car park entry point.

The Year 5-8 Learning Centre will open in July 2025, and once opened the secondary school enrolments are expected to increase from 962 students (2022) to around 1,144 students (+ 182). The primary school and ELC enrolment numbers will remain stable at around 558 and 98 children, respectively.

Staff numbers are expected to increase from 55 to around 61 primary school staff (+ 6) and 102 to around 132 secondary school staff (+30). The number of support staff and ELC staff are expected to remain at around 157 staff and 31 staff, respectively.

The 2025 student and staff numbers are estimations provided by the school for traffic assessment purposes. They are long term forecasts that are subject to annual fluctuations. Therefore, they should not be taken as definitive enrolment and staff numbers.

Once the Year 5-8 Learning Centre is constructed, the number of staff car parking spaces will reduce to 312 car parking spaces. No changes are proposed to the visitor parking (262 parking spaces), maintenance and trade vehicle parking (29 parking spaces) and loading and bus zones. Therefore, a total of 603 car parking spaces will be available on site for staff, visitor, and maintenance and trade vehicle parking.





4 PARKING ASSESSMENT

4.1 Car Parking

ADVERTISED PLAN

4.1.1 Car Parking Requirement

Clause 52.06 of the Wyndham Planning Scheme details the car parking requirement in accordance with State and Local Planning Policy for a proposed new use or a proposed change in existing use within Wyndham City Council. Clause 52.06-5, Table 1 sets out the number of car parking spaces required for a particular use.

As outlined in Section 3:

- Primary school staff numbers are expected to increase by around 6 staff (from 55 to around 61),
- Secondary school staff numbers are expected to increase by around 30 staff (from 102 to around 132).
- The number of support staff and ELC staff are expected to remain at around 157 staff and 31 staff, respectively.
- > It is proposed to remove 18 staff off-street car parking spaces from the site.

Table 4.1 provides details of the car parking requirement for the Early Learning Centre (ELC), Primary School and Secondary School use based on these increased staff numbers.

Table 4.1: Car Parking Requirement

Use	Rate & Measure	Quantity	No. of Spaces Required
ELC (Child Care Centre)	0.22 spaces to each child.	98 children	21
Primary School	1 space to each employee that is part of the maximum number of employees on the site at any time	61 employees	61
Secondary School	1.2 spaces to each employee that is part of the maximum number of employees on the site at any time	132 employees	158
Support Staff	1 space to each employee that is part of the maximum number of employees on the site at any time	157 employees	157
		Total	397

As detailed in Table 4.1, the school has a statutory car parking requirement equivalent to 397 car parking spaces which is made up of 21 car parking spaces for the ELC use and 376 car parking spaces for the primary and secondary school use.

Therefore, the provision of 603 car parking spaces (312 spaces for staff use, 29 spaces for maintenance and trade vehicle use, and 262 spaces for visitor use) exceeds the statutory car parking requirement by 206 car parking spaces.

4.1.2 Car Parking Demand Assessment

A car parking demand assessment based on the existing demand generated by the school site as detailed in Section 2.4.1.4 (0.17 visitor car parking spaces per student and 0.52 car parking spaces per staff member¹³) has also been completed to confirm the adequacy of the car parking supply for both visitors and staff.

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13 Student and staff numbers are based on the expected number of student and staff that will be on site during the PM peak as pick up time, like the existing conditions (85% students and 93% staff). Includes ELC enrollments and staff numbers, ments and staff numbers, ments and staff numbers, ments and staff numbers, ments and staff numbers.

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Based on an estimated total of 1,530 students (85%) and 354 staff (93%) being present on site at any one time¹³, the school is expected to generate a peak demand for car parking equivalent to 260 visitor car parking spaces and 184 staff car parking spaces, a total of 444 car parking spaces.

Therefore, the provision of 262 visitor car parking spaces and 312 staff car parking spaces, exceeds the expected peak visitor and staff car parking demand likely to be generated by the ELC, Primary School and Secondary School uses by 2 and 129 car parking spaces, respectively. Therefore, once the Year 5-8 Learning Centre is constructed and operational in 2025, the school is expected to have an oversupply of parking equivalent to 159 car parking spaces (includes the 29 spaces for maintenance and trade vehicle use).

ADVERTISED PLAN



4.2 Bicycle Facilities

Clause 52.34-5, Table 1 of the Wyndham Planning Scheme sets out the minimum number of bicycle spaces required for a for a proposed change in existing use within Wyndham City Council. For the proposed increase in secondary school student and staff numbers, the bicycle parking requirement is set out in Table 4.2.

Table 4.2: Bicycle Parking Requirement

Use	Rate & Measure	Quantity Change from 2022 to 2025	No. of Spaces Required
Drimon, Cobool	1 to each 20 employees		0
Primary School	1 to each 5 pupils over year 4	0 students	0
Canadam / Cabaal	1 to each 20 employees	+30 employees	2
Secondary School	1 to each 5 pupils	182 students	36
Support Staff	1 to each 20 employees	0 employees	0

As detailed in Table 4.2, the school has a statutory bicycle parking requirement to provide an extra 36 secondary school student bicycle parking spaces and 2 staff bicycle parking spaces.

Therefore, the school is required to provide the following bicycle infrastructure within the site:

- > 36 secondary school student bicycle parking spaces located centrally within the school site and at a bicycle rail, and
- > 2 staff bicycle parking spaces located close to the staff offices and within a bicycle locker or at a bicycle rail in a lockable compound.

All bicycle parking spaces are to be designed in accordance with Clause 52.34-6 of the Wyndham Planning Scheme.





5 TRAFFIC ASSESSMENT

A review of what impact the proposed increase in enrolment and staff numbers will have on the operational performance of the school access points in the future year (10 Year forecast period) follows.

5.1 Traffic Generation

Referring to Section 2.3.2.1, the school generates approximately 0.56 two-way vehicle trips per student (51% enter & 49% exit) during the 30 minute AM Peak period and 0.32 two-way vehicle trips per student (23% enter & 77% exit) during the 30 minute PM peak period.

Applying this traffic generation rate to the increase in student numbers (+182), it is expected that once the Year 5-8 Learning Centre is operational, the school will generate an extra 102 two-way vehicle trips per student (52 vehicles enter & 50 vehicles exit) during the 30 minute AM Peak period and 58 two-way vehicle trips per student (13 vehicles enter & 45 vehicles exit) during the 30 minute PM peak period.

Table 5.1 details the distribution of all new trips generated by the increase in enrolment and staff numbers at each access point which is based on the existing traffic volume distribution results. However, as the Zone A car park is close to capacity (99%¹⁴) during the PM Peak period, it is recommended that the school allow use of the Zone D & E car parks for secondary school student drop off & pick up, due to the availability of spaces (35 No.) within these car parks.

Therefore, all new trips that are expected to use the Gate C and D access points during the PM Peak period, will be distributed to the Doubell Gate. Further, due to the closure of the Zone I car park, all new trips exiting Gate C (staff trips) during both the AM and PM Peak period will be redistributed to Gate A.

	AM Peak		PM Peak	
Access Point	In	Out	In	Out
Doubell Gate	23	22	↑ 2 + 10 = 12	↑ 13 + 27 = 40
Main Gate	5	-	1	-
Gate A	-	↑ 1+ 1 = 2	-	↑ 2 + 3 = 5
Gate C	24	1 – 1 = 0	10 - 10 = 0	3 - 3 = 0
Gate D	-	26	-	27 – 27 = 0
Total	52	50	13	45

Table 5.1: AM & PM Peak Traffic Volume Distribution, Increase in Enrolments

5.2 Future Year Intersection Volumes

Refer Figure 5.1 for the future year (10 year forecast) traffic volumes at each access point, due to the expected growth in enrolment numbers.

As both Sayers Road and Marquands Road are close to capacity, and due to the major road upgrades to Leakes Road in the north, and Forsyth Road in the east, it is expected that there will be very minimal growth in the existing traffic volumes on both roads. Therefore, for modelling purposes an average annual traffic growth rate equivalent to 0.5% will be used for both roads.



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Westbourne Grammar School Year 5-8 Learning Centre Westbourne Grammar School

¹⁴ The two accessible car parking spaces for disabled users were the only vacant car parking spaces within the Zone A car park.



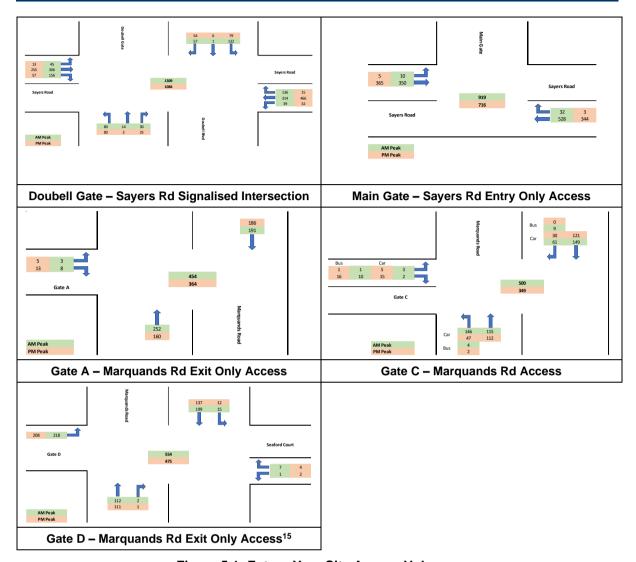


Figure 5.1: Future Year Site Access Volumes

5.3 Future Year Site Access Operational Performance

A check of the operational performance of the existing site access points with the future year traffic volumes was undertaken using SIDRA Intersection 9.1 software to understand what impact the increase in enrolment and staff numbers will have on the operational performance of the site access points.

A summary of the Degree of Saturation, Average Delay, Level of Service, and 95% Back of Queue Distance results of the analysis for the "total approach" of the intersection with the future year traffic volumes is provided in Table 2.3 and Table 2.4. Full results are provided in Appendix E.



¹⁵ Not all vehicles existing Gate D turned left out of the site as required.



Table 5.2: SIDRA Summary Results - Doubell Gate, Sayers Rd Signalised Intersection

Approach Leg	Doubell Blvd (South) Sayers Road (North) Doubell Blvd (North)		Sayers Road (West)	Total Intersection	
AM Peak					
Degree of Saturation	0.568	0.885	0.719	0.889	0.889
Average Delay (sec)	69.0	71.4	55.5	71.4	68.9
Level of Service	E	Е	Е	E	E
95% Queue Distance (m)	70.2	274.4	85.8	279.4	279.4
PM Peak					
Degree of Saturation	0.483	0.918	0.660	0.620	0.918
Average Delay (sec)	63.0	56.0	63.9	28.8	49.6
Level of Service	E	E	Е	С	D
95% Queue Distance (m)	60.6	401.5	63.8	126.4	401.5

The Doubell Gate, Sayers Road Signalised Intersection will continue to operate just below its capacity and within acceptable limits with a Level of Service E, Degree of Saturation below 0.92, Average Delay less than 69 seconds, and Queues less than 402 metres long during both the AM Peak and PM Peak periods. These results are very similar to the existing conditions.

Table 5.3: SIDRA Summary Results - Main Gate, Sayers Rd Entry Only Access

Approach Leg	Sayers Road (East)	Sayers Road (West)	Total Intersection
AM Peak			
Degree of Saturation	0.547	0.364	0.547
Average Delay (sec)	0.7	0.3	0.5
Level of Service	-	-	-
95% Queue Distance (m)	2.7	0	2.7
PM Peak			
Degree of Saturation	0.354	0.374	0.374
Average Delay (sec)	0.2	0.2	0.2
Level of Service	-	-	-
95% Queue Distance (m)	0.3	0	0.3



The Main Gate, Sayers Road Entry Only Access Intersection will continue to operate below its capacity with a Degree of Saturation below 0.60, Average Delay less than 1 second, and Queues less than 3 metre long during both the AM Peak and PM Peak periods. These results are very similar to the existing conditions.

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Table 5.4: SIDRA Summary Results - Gate A, Marquands Rd Exit Only Access

Approach Leg	Marquands Road (South)	Marquands Road (North)	School Access Road (West)	Total Intersection
AM Peak				
Degree of Saturation	0.261	0.203	0.069	0.261
Average Delay (sec)	0.1	0.1	5.0	0.2
Level of Service	-	-	А	-
95% Queue Distance (m)	0	0	0.7	0.7
PM Peak				
Degree of Saturation	0.167	0.200	0.041	0.200
Average Delay (sec)	0	0.1	3.5	0.2
Level of Service	-	-	А	-
95% Queue Distance (m)	0	0	0.9	0.9

The Gate A, Marquands Road Exit Only Access Intersection will continue to operate well below its capacity with a Degree of Saturation below 0.27, Average Delay less than 1 second, and Queues less than 1 metre long during both the AM Peak and PM Peak periods. These results are very similar to the existing conditions.

Table 5.5: SIDRA Summary Results - Gate C, Marquands Rd Access

Approach Leg	Marquands Road (South)	Marquands Road (North)	Williamstown Drive (West)	Total Intersection	
AM Peak					
Degree of Saturation	0.343	0.325	0.106	0.343	
Average Delay (sec)	3.4	4.7	10.7	4.2	
Level of Service	-	-	В	-	
95% Queue Distance (m)	14.1	12.9	3.7	14.1	
PM Peak ¹⁶					
Degree of Saturation	0.958	0.979	0.140	0.979	
Average Delay (sec)	5.9	6.3	5.3	6.0	
Level of Service	-	-	А	-	
95% Queue Distance (m)	64.2	18.8	4.4	64.2	

The Gate C, Marquands Road Access Intersection will continue to operate well below its capacity during the AM Peak period with a Degree of Saturation below 0.35, Average Delay less than 5 seconds, and Queues less than 15 metres long. Where-as during the PM Peak period, the Marquands Road Gate C Two-way Access Point will operate just below its capacity with a Degree of Saturation less than 0.98, Average Delay of around 6 seconds, and Queues up to 65 metres long.

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¹⁶ Capacity adjustment factor amended to consider that the schools children's crossing supervisor will consider queue lengths eview as within Williamstown Drive by stopping children from crossing the Zone A car park access point.



These results are an improvement to the existing conditions, and any capacity issues are associated with the expected growth in the Marquands Road through traffic volumes, and not due to the proposed development traffic i.e. growth in development traffic will be redistributed to the Doubell Gate.

Table 5.6: SIDRA Summary Results - Gate D, Marquands Rd Exit Only Access

Approach Leg	Marquands Road (South)	Seaford Circuit (East)	Marquands Road (North)	School Access Road (West)	Total Intersection
AM Peak					
Degree of Saturation	0.121	0.049	0.226	0.326	0.326
Average Delay (sec)	0.2	13.5	0.4	1.0	0.8
Level of Service	-	В	-	А	-
95% Queue Distance (m)	0.3	1.0	0	11.3	11.3
PM Peak					
Degree of Saturation	0.236	0.025	0.314	0.311	0.314
Average Delay (sec)	0.1	9.8	0.6	1.0	0.8
Level of Service	-	А	-	А	-
95% Queue Distance (m)	0.1	0.6	0	10.6	10.6

The Gate D, Marquands Road Exit Only Access Intersection will continue to operate well below its capacity with a Degree of Saturation below 0.40, Average Delay less than 1 second, and Queues less than 12 metres long during both the AM Peak and PM Peak periods. These results are very similar to the existing conditions.

The proposed increase in enrolment and staff numbers are not expected to have a detrimental impact on the existing site access points including Marquands Road and Sayers Road provided that the following recommendations are adopted:

- 1. The school allow use of the Zone D & E car parks for secondary school student drop off & pick up, due to the availability of 35 No. car parking spaces within these car parks.
- 2. The school's children's crossing supervisor monitor the Williamstown Drive vehicle queues, and if queuing traffic backs onto Marquands Road, then they are to stop children from crossing the Zone A car park access point, so that vehicles can enter the car park.





6 CONCLUSION

The proposal to construct a Year 5-8 Learning Centre within Westbourne Grammar School which includes the removal of 18 staff car parking spaces and circulation road within the Zone I car park, and the modification of the Zone H car park to include a turn area at its western end is considered adequate for the following reasons:

- The provision of 603 car parking spaces (312 spaces for staff use, 29 spaces for maintenance and trade vehicle use, and 262 spaces for visitor use) exceeds the statutory car parking requirement as set out in Clause 52.06 of the Wyndham Planning Scheme by 206 car parking spaces.
- The provision of 603 car parking spaces, exceeds the expected peak demand for car parking likely to be generated by the site by 159 car parking spaces.
- The provision of 36 secondary school student bicycle parking spaces and 2 staff bicycle parking spaces are in accordance with the statutory bicycle facilities requirement as set out in Clause 52.34 of the Wyndham Planning Scheme.
- The increase in vehicle trips to/ from the site will not have a detrimental impact on the operation of the existing site access points provided that the following recommendations are adopted:
 - The school allow use of the Zone D & E car parks for secondary school student drop off and pick up.
 - The school's children's crossing supervisor monitor the Williamstown Drive vehicle queues, to avoid vehicles queuing back onto Marquands Road.
- The closure of the north-south internal circulation road and minor changes to car parking is not anticipated to change site access patterns.

Therefore, provided that the above recommendations are adopted by Westbourne Grammar School, then there are no traffic engineering reasons as to why the responsible authority should not grant approval to amend the existing permit to include the construction of Year 5-8 Learning Centre, removal of 18 staff car parking spaces, and modification of the Zone H car park to include a turn area at its western end.





APPENDIX A - EXISTING SIDRA ANALYSIS RESULTS

ADVERTISED PLAN

SITE LAYOUT

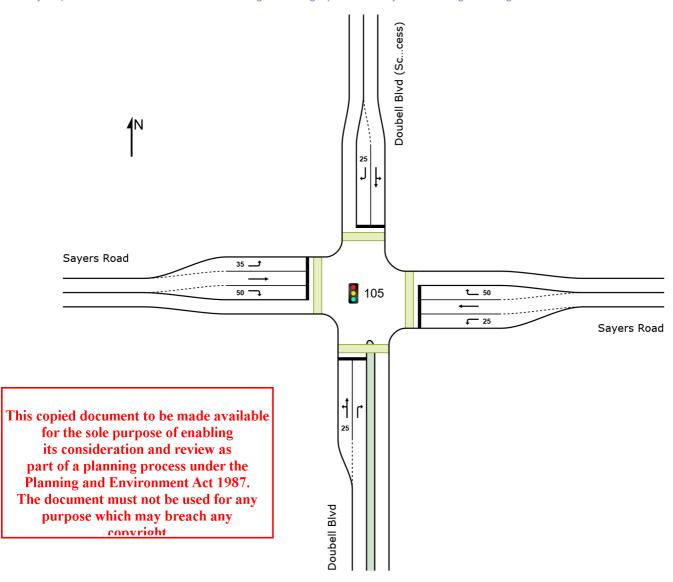
Site: 105 [380 Sayers Rd Access - AM Peak (Site Folder:

Existing Conditions)]

380 Sayers Road Access Site Category: Existing Design

Signals - EQUISAT (Fixed-Time/SCATS) Isolated

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



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LANE SUMMARY

Site: 105 [380 Sayers Rd Access - AM Peak (Site Folder:

Existing Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

380 Sayers Road Access Site Category: Existing Design

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 120 seconds (Site User-Given Cycle Time)

Variable Sequence Analysis applied. The results are given for the selected output sequence.

Lane Use	and P	erfor	mance												
	Dem Flo		Arrival		Сар.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Ba Que [Veh		Lane Config	Lane Length	Cap. P Adj. B	
	veh/h	%	veh/h	%	veh/h	v/c	%	sec		[VOII	m		m	%	%
South: Do	ubell Blv	∕d													
Lane 1	176	0.0	176	0.0	345 ¹	0.510	100	63.3	LOS E	9.2	64.7	Short	25	0.0	NA
Lane 2	58	0.0	58	0.0	175	0.331	100	72.2	LOS E	2.8	19.6	Full	500	0.0	0.0
Approach	234	0.0	234	0.0		0.510		65.5	LOS E	9.2	64.7				
East: Saye	ers Road	d													
Lane 1	74	0.0	74	0.0	907	0.082	100	48.6	LOS D	2.3	16.0	Short	25	0.0	NA
Lane 2	598	4.0	598	4.0	711 ¹	0.841	100	64.4	LOS E	33.5	242.9	Full	415	0.0	0.0
Lane 3	240	0.0	240	0.0	473	0.507	100	67.7	LOS E	7.0	48.8	Short	50	0.0	NA
Approach	912	2.6	912	2.6		0.841		64.0	LOS E	33.5	242.9				
North: Dou	ubell Blv	d (Sch	nool Acc	ess)											
Lane 1	234	0.0	234	0.0	539 ¹	0.434	100	43.6	LOS D	10.5	73.2	Full	500	0.0	0.0
Lane 2	100	0.0	100	0.0	173	0.580	100	66.5	LOS E	5.0	34.9	Short	25	0.0	NA
Approach	334	0.0	334	0.0		0.580		50.4	LOS D	10.5	73.2				
West: Say	ers Roa	ıd													
Lane 1	80	0.0	80	0.0	907	0.088	100	46.1	LOS D	2.5	17.3	Short	35	0.0	NA
Lane 2	582	8.0	582	8.0	695 ¹	0.837	100	61.7	LOS E	32.6	243.8	Full	500	0.0	0.0
Lane 3	296	1.0	296	1.0	469	0.631	100	74.3	LOS E	9.0	63.7	Short	50	0.0	NA
Approach	958	5.2	958	5.2		0.837		64.3	LOS E	32.6	243.8				
All Vehicles	2438	3.0	2438	3.0		0.841		62.4	LOS E	33.5	243.8				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Intersection and Approach LOS values are based on average delay for all lanes.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

1 Reduced capacity due to a short lane effect. Short lane queues may extend into the full-length lanes. Delay and stops experienced by drivers upstream of short lane entry have been accounted for.

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PHASING SUMMARY

Site: 105 [380 Sayers Rd Access - AM Peak (Site Folder:

Existing Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

380 Sayers Road Access Site Category: Existing Design

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 120 seconds (Site User-Given Cycle Time)

Variable Sequence Analysis applied. The results are given for the selected output sequence.

Timings based on settings in the Site Phasing & Timing dialog

Phase Times determined by the program Phase Sequence: Leading Right Turn

Input Phase Sequence: A, B, B2*, B3*, C, D, D2*, D3*

Output Phase Sequence: A, B, C, D

Reference Phase: Phase A

(* Variable Phase)

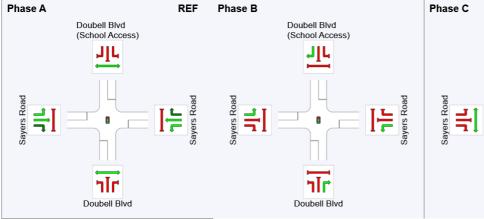
Phase Timing Summary

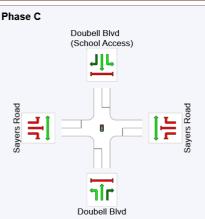
Phase	Α	В	С	D
Phase Change Time (sec)	0	55	68	91
Green Time (sec)	49	7	17	23
Phase Time (sec)	55	13	23	29
Phase Split	46%	11%	19%	24%
Phase Frequency (%)	100.0 ⁴	100.0 ⁴	100.0 ⁴	100.0 ⁴

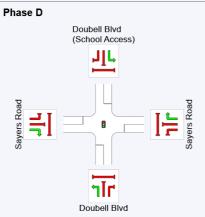
See the Timing Analysis report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Minor Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.

4 Phase Frequency specified by the user (phase times not specified).

Output Phase Sequence









REF: Reference Phase VAR: Variable Phase



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ADVERTISED PLAN

LANE SUMMARY

Site: 105 [380 Sayers Rd Access - PM Peak (Site Folder:

Existing Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

380 Sayers Road Access Site Category: Existing Design

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 120 seconds (Site User-Given Cycle Time)

Variable Sequence Analysis applied. The results are given for the selected output sequence.

Lane Use	and P	erfor	mance												
	Dem Flo	WS	Arrival		Cap.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% B: Que	eue	Lane Config	Lane Length	Cap. P Adj. B	
	[Total veh/h	HV] %	[Total veh/h	пv ј %	veh/h	v/c	%	sec		[Veh	Dist] m		m	%	%
South: Do	ubell Blv	/d													
Lane 1	156	0.0	156	0.0	354 ¹	0.441	100	58.2	LOS E	8.1	56.4	Short	25	0.0	NA
Lane 2	48	0.0	48	0.0	201	0.239	100	63.8	LOS E	2.4	16.5	Full	500	0.0	0.0
Approach	204	0.0	204	0.0		0.441		59.6	LOS E	8.1	56.4				
East: Saye	ers Road	d													
Lane 1	60	3.0	60	3.0	1148	0.052	100	34.2	LOS C	1.3	9.5	Short	25	0.0	NA
Lane 2	886	3.0	886	3.0	1017 ¹	0.872	100	48.6	LOS D	47.4	340.0	Full	415	0.0	0.0
Lane 3	18	0.0	18	0.0	466	0.039	100	47.6	LOS D	0.4	2.7	Short	50	0.0	NA
Approach	964	2.9	964	2.9		0.872		47.7	LOS D	47.4	340.0				
North: Dou	ubell Blv	d (Sch	nool Acc	ess)											
Lane 1	120	0.0	120	0.0	353	0.340	100	44.9	LOS D	6.1	42.7	Full	500	0.0	0.0
Lane 2	76	0.0	76	0.0	174	0.437	100	43.5	LOS D	3.8	26.6	Short	25	0.0	NA
Approach	196	0.0	196	0.0		0.437		44.3	LOS D	6.1	42.7				
West: Say	ers Roa	ıd													
Lane 1	16	0.0	16	0.0	1173	0.014	100	22.0	LOS C	0.3	2.4	Short	35	0.0	NA
Lane 2	486	2.0	486	2.0	1068 ¹	0.455	100	24.2	LOS C	16.6	117.9	Full	500	0.0	0.0
Lane 3	108	0.0	108	0.0	204	0.529	100	40.5	LOS D	3.7	26.1	Short	50	0.0	NA
Approach	610	1.6	610	1.6		0.529		27.0	LOS C	16.6	117.9				
All Vehicles	1974	1.9	1974	1.9		0.872		42.2	LOS D	47.4	340.0				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Intersection and Approach LOS values are based on average delay for all lanes.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

1 Reduced capacity due to a short lane effect. Short lane queues may extend into the full-length lanes. Delay and stops experienced by drivers upstream of short lane entry have been accounted for.

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PHASING SUMMARY

Site: 105 [380 Sayers Rd Access - PM Peak (Site Folder:

Existing Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

380 Sayers Road Access Site Category: Existing Design

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 120 seconds (Site User-Given Cycle Time)

Variable Sequence Analysis applied. The results are given for the selected output sequence.

Timings based on settings in the Site Phasing & Timing dialog

Phase Times determined by the program Phase Sequence: Leading Right Turn

Input Phase Sequence: A, B, B2*, B3*, C, D, D2*, D3*

Output Phase Sequence: A, B, C, D

Reference Phase: Phase A

(* Variable Phase)

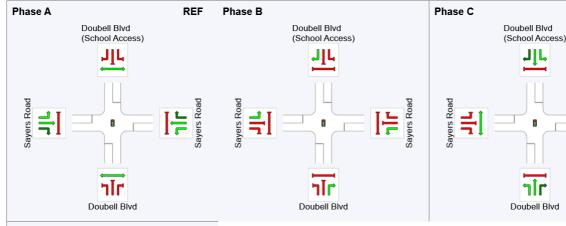
Phase Timing Summary

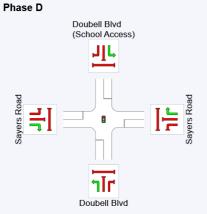
Phase	Α	В	С	D
Phase Change Time (sec)	0	73	85	108
Green Time (sec)	67	6	17	6
Phase Time (sec)	73	12	23	12
Phase Split	61%	10%	19%	10%
Phase Frequency (%)	100.0 ⁴	100.0 ⁴	100.0 ⁴	100.0 ⁴

See the Timing Analysis report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Minor Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.

4 Phase Frequency specified by the user (phase times not specified).

Output Phase Sequence







REF: Reference Phase VAR: Variable Phase



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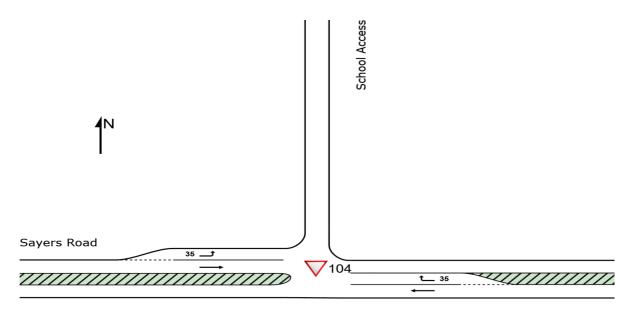
SITE LAYOUT

▽ Site: 104 [300 Sayers Rd Access - AM Peak (Site Folder:

Existing Conditions)]

300 Sayers Road Access Site Category: Existing Design Give-Way (Two-Way)

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



Sayers Road

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V Site: 104 [300 Sayers Rd Access - AM Peak (Site Folder:

Existing Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

300 Sayers Road Access Site Category: Existing Design Give-Way (Two-Way)

Laura Hay															
Lane Use	and P Dem Flo	and	mance Arrival		Сар.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% B: Que		Lane Config	Lane Length	Cap. P Adj. B	
	[Total veh/h	HV] %	[Total veh/h	HV] %	veh/h	v/c	%	sec		[Veh	Dist] m		m	%	%
East: Saye	ers Roa	d													
Lane 1 Lane 2	1004 56	3.2 0.0	1004 56	3.2 0.0	1930 624	0.520 0.090	100 100	0.1 9.9	LOS A LOS A	0.0 0.3	0.0 2.3	Full Short	180 35	0.0 0.0	0.0 NA
Approach	1060	3.0	1060	3.0		0.520		0.6	NA	0.3	2.3				
West: Say	ers Roa	ıd													
Lane 1	18	0.0	18	0.0	1828	0.010	100	5.6	LOSA	0.0	0.0	Short	35	0.0	NA
Lane 2	666	3.9	666	3.9	1922	0.347	100	0.1	LOS A	0.0	0.0	Full	415	0.0	0.0
Approach	684	3.8	684	3.8		0.347		0.2	NA	0.0	0.0				
All Vehicles	1744	3.3	1744	3.3		0.520		0.5	NA	0.3	2.3				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: 104 [300 Sayers Rd Access - PM Peak (Site Folder:

Existing Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

300 Sayers Road Access Site Category: Existing Design Give-Way (Two-Way)

Lane Use	e and F	erfor	mance												
	Dem Flo [Total	WS	Arrival		Сар.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Ba Que [Veh	eue	Lane Config	Lane Length	Cap. F Adj. B	
	veh/h	пv ј %	[Total veh/h	пv ј %	veh/h	v/c	%	sec		[veii	Dist] m		m	%	%
East: Say	ers Roa	d													
Lane 1	654	2.1	654	2.1	1944	0.336	100	0.0	LOSA	0.0	0.0	Full	180	0.0	0.0
Lane 2	6	0.0	6	0.0	609	0.010	100	9.7	LOS A	0.0	0.2	Short	35	0.0	NA
Approach	660	2.1	660	2.1		0.336		0.1	NA	0.0	0.2				
West: Say	ers Roa	ıd													
Lane 1	8	0.0	8	0.0	1828	0.004	100	5.6	LOS A	0.0	0.0	Short	35	0.0	NA
Lane 2	694	1.4	694	1.4	1953	0.355	100	0.1	LOS A	0.0	0.0	Full	415	0.0	0.0
Approach	702	1.4	702	1.4		0.355		0.2	NA	0.0	0.0				
All Vehicles	1362	1.7	1362	1.7		0.355		0.1	NA	0.0	0.2				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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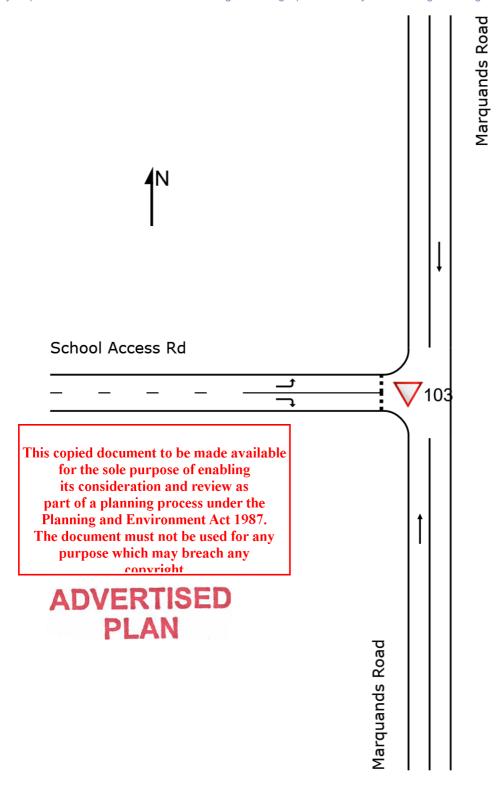
SITE LAYOUT

V Site: 103 [Marquands Rd Gate A - AM Peak (Site Folder:

Existing Conditions)]

Gate A Access Site Category: Existing Design Give-Way (Two-Way)

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



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ADVERTISED PLAN

∇ Site: 103 [Marquands Rd Gate A - AM Peak (Site Folder:

Existing Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Gate A Access

Site Category: Existing Design

Give-Way (Two-Way)

Lane Use	and P	erfori	mance												
	Dem Flo	WS	Arrival		Сар.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	Qu	ack Of eue	Lane Config	Lane Length	Cap. P Adj. B	
	[Total veh/h	HV]	[Total veh/h	HV] %	veh/h	v/c	%	sec		[Veh	Dist] m		m	%	%
South: Ma	rquands	Road													
Lane 1	480	2.9	480	2.9	1934	0.248	100	0.1	LOS A	0.0	0.0	Full	500	0.0	0.0
Approach	480	2.9	480	2.9		0.248		0.1	NA	0.0	0.0				
North: Ma	rquands	Road													
Lane 1	364	7.1	364	7.1	1883	0.193	100	0.1	LOS A	0.0	0.0	Full	500	0.0	0.0
Approach	364	7.1	364	7.1		0.193		0.1	NA	0.0	0.0				
West: Sch	ool Acce	ess Rd													
Lane 1	4	0.0	4	0.0	1014	0.004	100	1.7	LOS A	0.0	0.1	Full	500	0.0	0.0
Lane 2	12	0.0	12	0.0	520	0.023	100	5.7	LOSA	0.1	0.5	Full	500	0.0	0.0
Approach	16	0.0	16	0.0		0.023		4.7	LOSA	0.1	0.5				
All Vehicles	860	4.6	860	4.6		0.248		0.1	NA	0.1	0.5				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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∇ Site: 103 [Marquands Rd Gate A - PM Peak (Site Folder:

Existing Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Gate A Access

Site Category: Existing Design

Give-Way (Two-Way)

Lane Use	and P	erfori	mance												
	Dem Flo	WS	Arrival		Сар.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	Que	ack Of eue	Lane Config	Lane Length	Cap. F Adj. B	
	[Total veh/h	HV] %	[Total veh/h	HV] %	veh/h	v/c	%	sec		[Veh	Dist] m		m	%	%
South: Ma	rquands	Road													
Lane 1	304	4.6	304	4.6	1913	0.159	100	0.0	LOS A	0.0	0.0	Full	500	0.0	0.0
Approach	304	4.6	304	4.6		0.159		0.0	NA	0.0	0.0				
North: Ma	rquands	Road													
Lane 1	354	9.0	354	9.0	1861	0.190	100	0.1	LOS A	0.0	0.0	Full	500	0.0	0.0
Approach	354	9.0	354	9.0		0.190		0.1	NA	0.0	0.0				
West: Sch	ool Acc	ess Rd													
Lane 1	8	0.0	8	0.0	1233	0.006	100	1.0	LOS A	0.0	0.2	Full	500	0.0	0.0
Lane 2	20	0.0	20	0.0	658	0.030	100	4.2	LOSA	0.1	0.7	Full	500	0.0	0.0
Approach	28	0.0	28	0.0		0.030		3.2	LOSA	0.1	0.7				
All Vehicles	686	6.7	686	6.7		0.190		0.2	NA	0.1	0.7				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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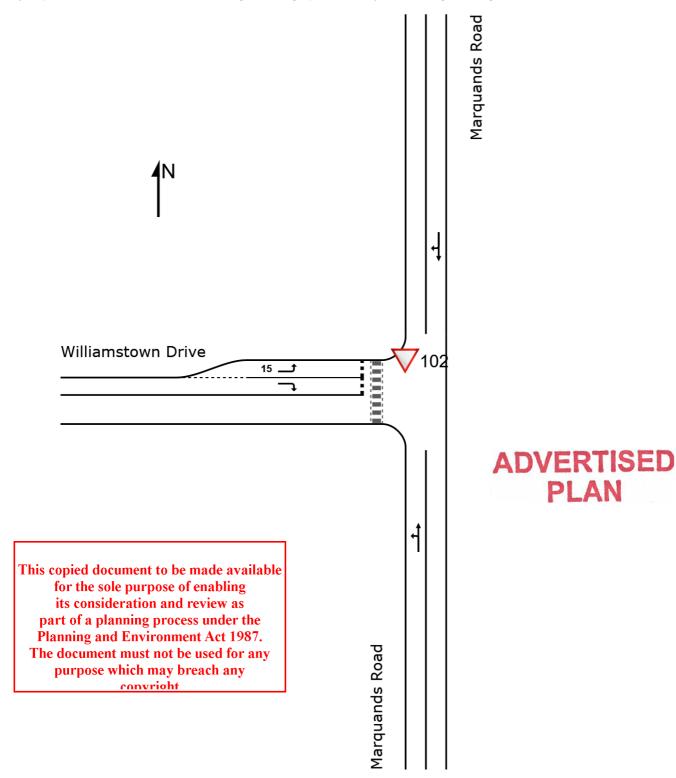
SITE LAYOUT

▽ Site: 102 [Marquands Rd Gate C - AM Peak (Site Folder:

Existing Conditions)]

Gate D Access Site Category: Existing Design Give-Way (Two-Way)

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



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ADVERTISED PLAN

V Site: 102 [Marquands Rd Gate C - AM Peak (Site Folder:

Existing Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Gate D Access

Site Category: Existing Design

Give-Way (Two-Way)

Lane Use	e and F	erfori	mance												
	Dem Flo	WS	Arrival		Сар.	Deg. Satn	Lane Util.		Level of Service	95% B	eue	Lane Config	Lane Length	Cap. P Adj. B	
	[Total veh/h	HV] %	[Total veh/h	HV] %	veh/h	v/c	%	sec		[Veh	Dist] m		m	%	%
South: Ma	rquands	s Road													
Lane 1	484	3.0	484	3.0	1549	0.312	100	3.3	LOS A	1.7	12.3	Full	500	0.0	0.0
Approach	484	3.0	484	3.0		0.312		3.3	NA	1.7	12.3				
North: Ma	rquands	Road													
Lane 1	410	5.4	410	5.4	1399	0.293	100	4.1	LOS A	1.4	10.5	Full	500	0.0	0.0
Approach	410	5.4	410	5.4		0.293		4.1	NA	1.4	10.5				
West: Will	iamstow	n Driv	е												
Lane 1	8	3.3	8	3.3	1254	0.006	100	8.0	LOS A	0.0	0.2	Short	15	0.0	NA
Lane 2	24	83.3	24	83.3	254	0.095	100	12.1	LOS B	0.3	3.3	Full	500	0.0	0.0
Approach	32	63.3	32	63.3		0.095		9.3	LOSA	0.3	3.3				
All Vehicles	926	6.1	926	6.1		0.312		3.9	NA	1.7	12.3				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: 102 [Marquands Rd Gate C - PM Peak (Site Folder:

Existing Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Gate C Access

Site Category: Existing Design

Give-Way (Two-Way)

Lane Us	e and F	erfor	mance	!											
	Dem Flo [Total	WS	Arrival [Total		Сар.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% B Que [Veh		Lane Config	Lane Length	Cap. F Adj. E	Prob. Block.
	veh/h	% -	veh/h	%	veh/h	v/c	%	sec		<u> </u>	m ¹		m	%	%
South: Ma	rquands	s Road													
Lane 1	312	4.2	312	4.2	314	0.993	100	10.4	LOS B	14.0	101.5	Full	500	-81.3	0.0
Approach	312	4.2	312	4.2		0.993		10.4	NA	14.0	101.5				
North: Ma	rquands	Road													
Lane 1	290	5.1	290	5.1	254	1.140	100	69.0	LOS F	14.0	102.3	Full	500	-85.2	0.0
Approach	290	5.1	290	5.1		1.140		69.0	NA	14.0	102.3				
West: Will	iamstow	n Driv	е												
Lane 1	12	16.7	12	16.7	1152	0.010	100	1.0	LOS A	0.0	0.3	Short	15	0.0	NA
Lane 2	62	51.6	62	51.6	459	0.135	100	5.8	LOS A	0.4	4.3	Full	500	0.0	0.0
Approach	74	45.9	74	45.9		0.135		5.0	LOS A	0.4	4.3				
All Vehicles	676	9.1	676	9.1		1.140		34.9	NA	14.0	102.3				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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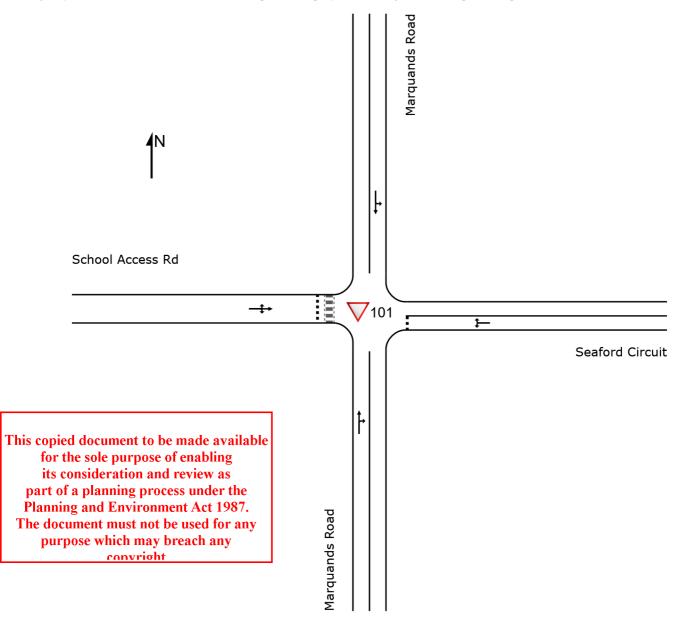
SITE LAYOUT

▽ Site: 101 [Marquands Rd Gate D - AM Peak (Site Folder:

Existing Conditions)]

Gate D Access Site Category: Existing Design Give-Way (Two-Way)

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V Site: 101 [Marquands Rd Gate D - AM Peak (Site Folder:

Existing Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Gate D Access

Site Category: Existing Design

Give-Way (Two-Way)

Lane Use	and P	erfori	mance												
	Dem Flov [Total veh/h	WS	Arrival [Total veh/h		Cap.	Deg. Satn v/c	Lane Util. %	Aver. Delay sec	Level of Service	95% Ba Que [Veh		Lane Config	Lane Length m	Cap. F Adj. B %	
South: Ma	rquands	Road													
Lane 1	218	5.2	218	5.2	1884	0.116	100	0.2	LOSA	0.0	0.3	Full	500	0.0	0.0
Approach	218	5.2	218	5.2		0.116		0.2	NA	0.0	0.3				
East: Seaf	ord Circ	uit													
Lane 1	16	0.0	16	0.0	386	0.041	100	11.8	LOS B	0.1	0.9	Full	500	0.0	0.0
Approach	16	0.0	16	0.0		0.041		11.8	LOS B	0.1	0.9				
North: Mar	quands	Road													
Lane 1	406	5.4	406	5.4	1897	0.214	100	0.4	LOSA	0.0	0.0	Full	500	0.0	0.0
Approach	406	5.4	406	5.4		0.214		0.4	NA	0.0	0.0				
West: Sch	ool Acce	ess Rd													
Lane 1	384	0.0	384	0.0	1105	0.347	100	1.9	LOSA	1.7	11.9	Full	500	0.0	0.0
Approach	384	0.0	384	0.0		0.347		1.9	LOSA	1.7	11.9				
All Vehicles	1024	3.2	1024	3.2		0.347		1.1	NA	1.7	11.9				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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▽ Site: 101 [Marquands Rd Gate D - PM Peak (Site Folder:

Existing Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Gate D Access

Site Category: Existing Design

Give-Way (Two-Way)

Lane Use	and P	erfor	mance												
	Dem Flov [Total	WS	Arrival [Total		Сар.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Ba Que [Veh		Lane Config	Lane Length	Cap. F Adj. B	llock.
	veh/h	%	veh/h	%	veh/h	v/c	%	sec			m		m	%	%
South: Ma	rquands	Road													
Lane 1	214	7.4	214	7.4	1872	0.114	100	0.1	LOSA	0.0	0.1	Full	500	0.0	0.0
Approach	214	7.4	214	7.4		0.114		0.1	NA	0.0	0.1				
East: Seaf	ord Circ	uit													
Lane 1	12	0.0	12	0.0	523	0.023	100	9.3	LOSA	0.1	0.5	Full	500	0.0	0.0
Approach	12	0.0	12	0.0		0.023		9.3	LOSA	0.1	0.5				
North: Mar	quands	Road													
Lane 1	282	0.7	282	0.7	1953	0.144	100	0.5	LOSA	0.0	0.0	Full	500	0.0	0.0
Approach	282	0.7	282	0.7		0.144		0.5	NA	0.0	0.0				
West: Sch	ool Acce	ess Rd													
Lane 1	416	0.0	416	0.0	1191	0.349	100	1.5	LOSA	1.7	11.9	Full	500	0.0	0.0
Approach	416	0.0	416	0.0		0.349		1.5	LOS A	1.7	11.9				
All Vehicles	924	1.9	924	1.9		0.349		1.0	NA	1.7	11.9				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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APPENDIX B – CAR PARKING OCCUPANCY SURVEY RESULTS

ADVERTISED PLAN



3 Trevethic Road Springvale Vic 3171 T:03 9888 1119 F:03 9548 5848 E: seng@ntsurveys.com.au

Survey Ref No:	6333
Job Name:	Westbourne Grammar School
	300 Sayers Rd
Suburb:	Truganina
Map Reference:	
Commonto:	

	Fri, 20-May-22
Survey StartTime:	
Survey End Time:	4:00 PM
Survey Interval:	0:15

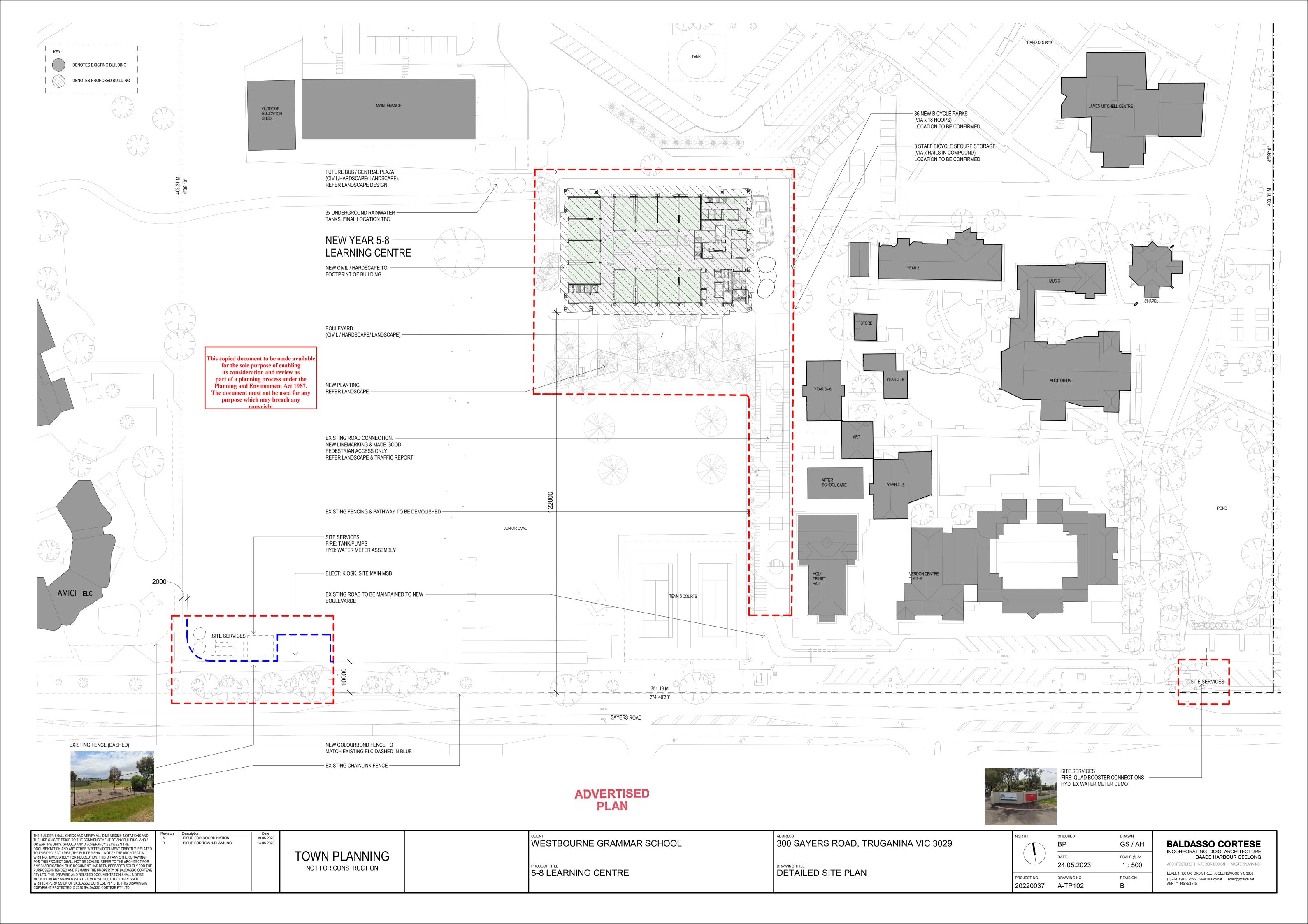
																Parking Occur	pancy								
Area	Street	Section	Side	Туре	Restrictions	Supply	Check	8:00 AM	8:15 AM	8:30 AM	8:45 AM	9:00 AM	9:15 AM	9:30 AM		10:00 AM		2:15 PM	2:30 PM	2:45 PM	3:00 PM	3:15 PM	3:30 PM	3:45 PM	4:00 PM
Α	Williamstown Drive	Westbourne Grammar School Car Park	-	Unrestricted		139	139	3	12	14	8	3	2	2	2	2	7	5	7	16	36	64	129	38	21
Α	Williamstown Drive	Westbourne Grammar School Car Park	-	Disabled Only		2	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Α	Williamstown Drive	Westbourne Grammar School Car Park	-	Kiss & Go Stay With Your Car 2minute Limit		9	9	0	6	8	4	1	0	0	0	0	1	1	8	9	9	9	9	4	3
Q	Marquands Road	Westbourne Grammar School Car Park	-	Unrestricted - Staff Parking		10	10	0	0	0	0	0	0	1	1	1	1	1	1	0	0	0	0	1	1
F	Sayers Road Service Rd	Westbourne Grammar School Car Park	-	Staff Parking		10	10	2	3	4	4	4	6	6	6	6	5	5	5	5	5	5	4	4	4
F	Sayers Road Service Rd	Westbourne Grammar School Car Park	-	Unrestricted - Staff Parking		17	17	3	6	9	11	11	13	13	12	12	14	14	14	13	14	15	15	14	7
G	Sayers Road Service Rd	Westbourne Grammar School Car Park	-	Unrestricted - Staff Parking		50	50	16	18	34	35	34	33	34	34	34	32	32	32	31	31	31	25	26	21
G	Sayers Road Service Rd	Westbourne Grammar School Car Park	-	Disabled Only		2	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
G	Sayers Road Service Rd	Westbourne Grammar School Car Park	-	Principle		1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
G	Sayers Road Service Rd	Westbourne Grammar School Car Park	-	Principle PA		1	1	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
G	Sayers Road Service Rd	Westbourne Grammar School Car Park	<u> </u>	Associate Principle		1	1	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
G	Sayers Road Service Rd	Westbourne Grammar School Car Park	-	Director Development		1	1	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	0
G	Sayers Road Service Rd	Westbourne Grammar School Car Park	<u> </u>	Head Of Admissions		1	1	0	0	1	1	1	1	1	1	1	0	0	0	0	0	0	0	0	0
G	Sayers Road Service Rd	Westbourne Grammar School Car Park	-	Director People Strategy Culture		1	1	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
Н		Westbourne Grammar School Car Park	-	Reserved - Staff Parking		4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	3	3	3	3	0
Н		Westbourne Grammar School Car Park	-	Unrestricted - Staff or Parents with Exemption		7	7	9	5	4	7	9	5	5	6	8	4	4	4	5	4	4	4	3	2
H		Westbourne Grammar School Car Park		Staff Parking		15	15	3	3	4	4	5	5	5	5	5	5	5	5	5	6	6	5	5	5
Н		Westbourne Grammar School Car Park	-	Disabled Only		1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
H		Westbourne Grammar School Car Park		Head of Junior School		1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Н		Westbourne Grammar School Car Park		Deputy Head of Junior School		1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
H		Westbourne Grammar School Car Park	-	Head of Verdon Campus		1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
H		Westbourne Grammar School Car Park	-	Excutive Sipport Officer		1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
		Westbourne Grammar School Car Park		Staff Parking		18	18	8	8	8	8	8	8	8	8	8	10	10	10	9	10	10	8	7	6
		Westbourne Grammar School Car Park	-	No Parking		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
J		Westbourne Grammar School Car Park		Staff Parking		30	30	16	17	17	17	17	17	17	16	16	12	12	12	12	12	12	11	9	8
K		Westbourne Grammar School Car Park		Unrestricted - Staff Parking		26	26	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0
_ L	1400	Westbourne Grammar School Car Park		Staff Parking		29	29	16	17	17	19	21	22	21	21	21	20	20	20	19	19	18	15	15	14
M	Williamstown Drive	Westbourne Grammar School Car Park	<u> </u>	Staff Parking	-	22	22	6	16	16	16	16	17	17	17	17	18	18	18	18	20	14	12	8	8
	Marquands Road	Westbourne Grammar School Car Park	_	Trade Vehicles or Deliveries Only		9	9	2	4	4	4	4	4	4	4	4	2	2	2	1	0	0	0	0	1
K		Westbourne Grammar School Car Park	-	Reserved - Staff Camp		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
K		Westbourne Grammar School Car Park	-	Disabled Only		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
N		Westbourne Grammar School Car Park	<u> </u>	Staff Parking		33	33	6	6	10	9	7	7	7	6	6	9	10	11	11	9	9	5	4	3
				Staff - Health Centre		1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Р		Westbourne Grammar School Car Park	_	Unrestricted - Maintenance & Property		9	9	4	4	5	5	5	5	5	5	4	5	5	5	5	2	2	2	2	2
P		Westbourne Grammar School Car Park	<u> </u>	Reserved Maintenance & Property		6	6	2	2	2	2	2	2	2	2	3	1	2	2	2	2	2	2	1	1
P		Westbourne Grammar School Car Park		Disabled Only - Staff		1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
D		Westbourne Grammar School Car Park	<u> </u>	Unrestricted - Visitor Weekend/ After Hours		40	40	5	5	6	5	5	5	5	5	5	4	4	4	6	5	9	10	3	1
E		Westbourne Grammar School Car Park		Unrestricted - Visitor Weekend/ After Hours		5	5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
С		Westbourne Grammar School Car Park		Kiss & Go Stay With Your Car 2minute Limit		12	12	5	2	8	7	0	0	0	0	0	0	0	1	13	13	13	13	5	0
С		Westbourne Grammar School Car Park		Unrestricted		29	29	4	8	17	11	13	2	2	2	2	2	3	3	12	29	29	29	2	2
В		Westbourne Grammar School Car Park		10minute Parking		24	24	10	12	17	18	19	17	17	17	18	19	20	20	20	23	30	27	17	13
В		Westbourne Grammar School Car Park		Disabled Only		2	2	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1	1	0	1
0		Westbourne Grammar School Car Park	<u> </u>	Unrestricted - Staff		9	9	7	7	8	8	8	8	8	8	8	7	6	6	6	5	5	5	5	5
0		Westbourne Grammar School Car Park	F-	Disabled Only - Staff		1	1	1 1	1 1	1	1	1	1	1	1	1	0	0	0	0	0	0	0	1 11	0
0		Westbourne Grammar School Car Park	F -	Unrestricted - Staff		33	33	11	11	11	11	11	11	11	13	13	14	14	14	14	14	14	14	14	14
0	1400 / D:	Westbourne Grammar School Car Park		Reserved - Staff (Amici/Winjeel & Year 9)		2	2	1	1	1	1	1	1	1	1	1	0	0	0	0	0	0	0	0	0
A	Williamstown Drive	Westbourne Grammar School Car Park	<u> </u>	Emergency Vehicles Parking Only		1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
G	Sayers Road Service Rd	Westbourne Grammar School Car Park		Loading Zone		2	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
H		Westbourne Grammar School Car Park		Loading Zone		2	2	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0
R		Westbourne Grammar School Car Park	-	Bus Parking Only		18	18	2	0	0	0	0	0	0	0	0	4	4	4	1	2	12	15	0	0
Р		Westbourne Grammar School Car Park	-	Bus Parking Only	ļ , <u>, , , , , , , , , , , , , , , , , ,</u>	3	3	0	0	0	0	0	0	0	0	0	1	1	2	2	3	3	3	0	0
				Totals (Excl. Bus, Emergency Vehicle&	Loading ∠one)		617	146	184	236	226	217	203	203	203	206	202	203	214	243	278	311	354	197	148
						Availab	ole Spaces		433	381	391	400	414	414	414	411	415	414	403	374	339	306	263	420	469
							Capacity	617	617	617	617	617	617	617	617	617	617	617	617	617	617	617	617	617	617

ADVERTISED PLAN



APPENDIX C - PROPOSED DETAILED SITE PLAN

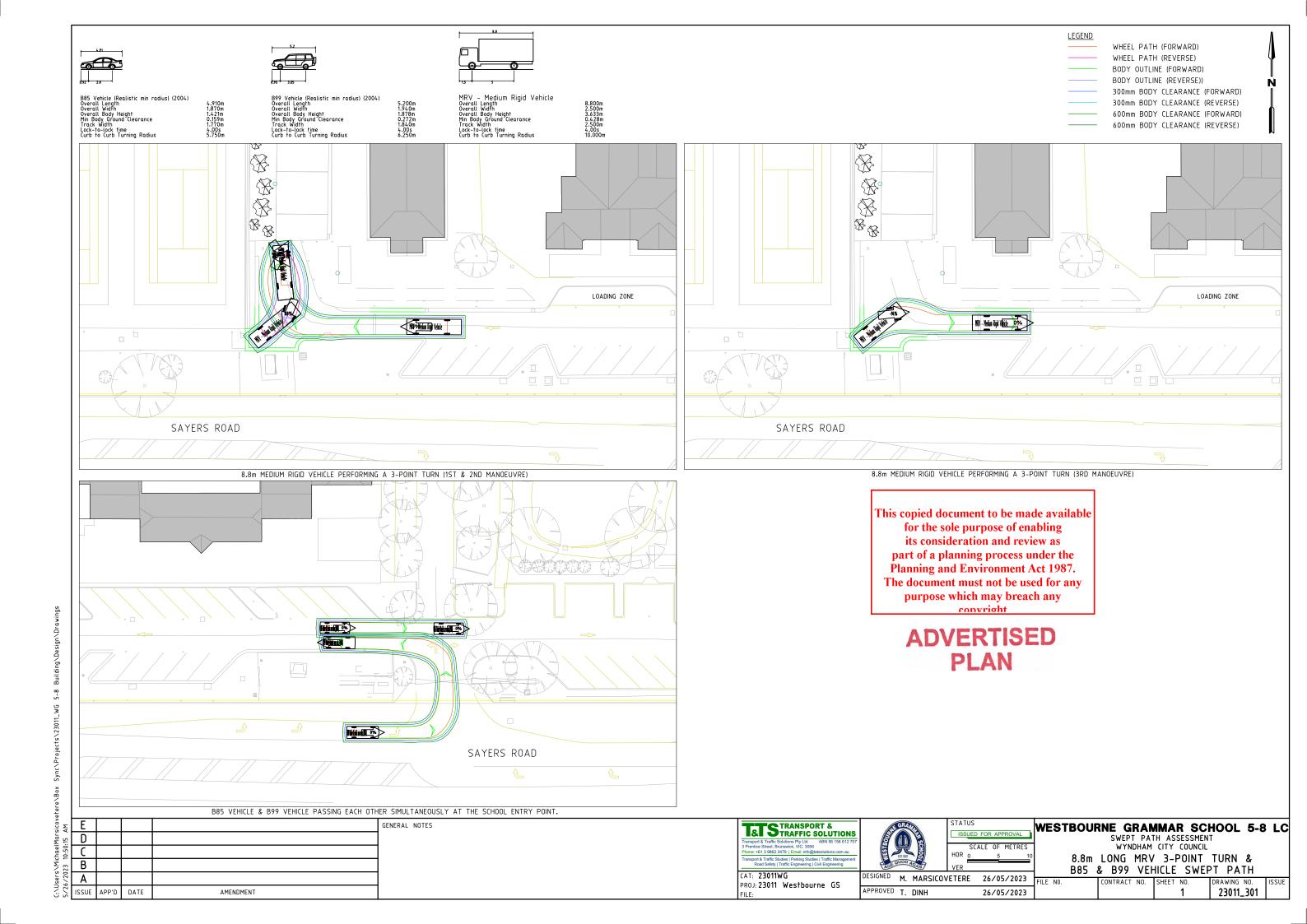
ADVERTISED PLAN





APPENDIX D - SWEPT PATH ASSESSMENT DRAWING

ADVERTISED PLAN





APPENDIX E - FUTURE YEAR SIDRA ANALYSIS RESULTS

ADVERTISED PLAN

Site: 105 [380 Sayers Rd Access - Future AM Peak (Site

Folder: Future Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

380 Sayers Road Access Site Category: Existing Design

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 120 seconds (Site User-Given Cycle Time)

Variable Sequence Analysis applied. The results are given for the selected output sequence.

Lane Use	and P	erfor	mance												
	Dem Flo		Arrival [Total		Сар.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% B Que [Veh		Lane Config	Lane Length	Cap. P Adj. B	
	veh/h	%	veh/h	%	veh/h	v/c	%	sec		[***	m		m	%	%
South: Do	ubell Blv	∕d													
Lane 1	188	0.0	188	0.0	331 ¹	0.568	100	66.4	LOS E	10.0	70.2	Short	25	0.0	NA
Lane 2	60	0.0	60	0.0	175	0.342	100	77.0	LOS E	2.9	20.3	Full	500	0.0	0.0
Approach	248	0.0	248	0.0		0.568		69.0	LOS E	10.0	70.2				
East: Saye	rs Road	d													
Lane 1	78	0.0	78	0.0	907	0.086	100	49.8	LOS D	2.4	16.9	Short	25	0.0	NA
Lane 2	628	4.0	628	4.0	710 ¹	0.885	100	70.8	LOS E	37.9	274.4	Full	415	0.0	0.0
Lane 3	272	0.0	272	0.0	453	0.600	100	78.8	LOS E	8.8	61.6	Short	50	0.0	NA
Approach	978	2.6	978	2.6		0.885		71.4	LOS E	37.9	274.4				
North: Dou	ibell Blv	d (Sch	nool Acc	ess)											
Lane 1	266	0.0	266	0.0	525 ¹	0.507	100	47.6	LOS D	12.3	85.8	Full	500	0.0	0.0
Lane 2	114	0.0	114	0.0	159	0.719	100	74.2	LOS E	5.9	41.0	Short	25	0.0	NA
Approach	380	0.0	380	0.0		0.719		55.5	LOS E	12.3	85.8				
West: Say	ers Roa	ıd													
Lane 1	90	0.0	90	0.0	907	0.099	100	47.7	LOS D	2.8	19.6	Short	35	0.0	NA
Lane 2	612	8.0	612	8.0	689 ¹	0.889	100	69.4	LOS E	37.4	279.4	Full	500	0.0	0.0
Lane 3	312	1.0	312	1.0	452	0.691	100	82.1	LOS F	11.3	79.8	Short	50	0.0	NA
Approach	1014	5.1	1014	5.1		0.889		71.4	LOS E	37.4	279.4				
All Vehicles	2620	2.9	2620	2.9		0.889		68.9	LOS E	37.9	279.4				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Intersection and Approach LOS values are based on average delay for all lanes.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

1 Reduced capacity due to a short lane effect. Short lane queues may extend into the full-length lanes. Delay and stops experienced by drivers upstream of short lane entry have been accounted for.

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PHASING SUMMARY

Site: 105 [380 Sayers Rd Access - Future AM Peak (Site

Folder: Future Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

380 Sayers Road Access Site Category: Existing Design

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 120 seconds (Site User-Given Cycle Time)

Variable Sequence Analysis applied. The results are given for the selected output sequence.

Timings based on settings in the Site Phasing & Timing dialog

Phase Times determined by the program Phase Sequence: Leading Right Turn

Input Phase Sequence: A, B, B2*, B3*, C, D, D2*, D3*

Output Phase Sequence: A, B, C, D

Reference Phase: Phase A

(* Variable Phase)

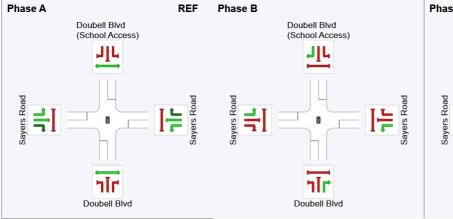
Phase Timing Summary

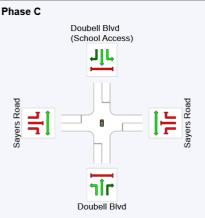
Phase	Α	В	С	D
Phase Change Time (sec)	0	55	68	91
Green Time (sec)	49	7	17	23
Phase Time (sec)	55	13	23	29
Phase Split	46%	11%	19%	24%
Phase Frequency (%)	100.0 ⁴	100.0 ⁴	100.0 ⁴	100.0 ⁴

See the Timing Analysis report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Minor Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.

4 Phase Frequency specified by the user (phase times not specified).

Output Phase Sequence





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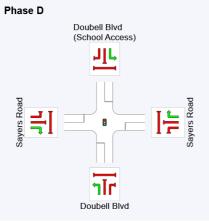
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REF: Reference Phase VAR: Variable Phase



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ADVERTISED PLAN

Site: 105 [380 Sayers Rd Access - Future PM Peak (Site

Folder: Future Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

380 Sayers Road Access Site Category: Existing Design

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 120 seconds (Site User-Given Cycle Time)

Variable Sequence Analysis applied. The results are given for the selected output sequence.

Lane Use	and P	erfor	mance												
	Dem Flo		Arrival	Flows	Сар.	Deg. Satn	Lane Util.	Aver. Delav	Level of Service	95% B: Que		Lane Config	Lane Length	Cap. F Adj. B	
	[Total	HV]	[Total							[Veh	Dist]				
South: Do	veh/h	/d	veh/h	%	veh/h	v/c	%	sec			m		m	%	%
Lane 1	166	0.0	166	0.0	344 ¹	0.483	100	61.1	LOS E	8.7	60.6	Short	25	0.0	NA
Lane 1	50	0.0	50	0.0	344 147	0.463	100	69.6	LOS E	0.7 2.5	17.2	Full	500	0.0	0.0
	216	0.0	216	0.0	147	0.339	100		LOSE	8.7	60.6	Full	500	0.0	0.0
Approach	210	0.0	210	0.0		0.483		63.0	LUS E	8.7	60.6				
East: Saye	ers Road	b													
Lane 1	64	3.0	64	3.0	1148	0.056	100	35.6	LOS D	1.4	10.2	Short	25	0.0	NA
Lane 2	932	3.0	932	3.0	1015 ¹	0.918	100	57.5	LOS E	55.9	401.5	Full	415	0.0	<mark>2.0</mark>
Lane 3	30	0.0	30	0.0	443	0.068	100	51.2	LOS D	0.7	4.6	Short	50	0.0	NA
Approach	1026	2.9	1026	2.9		0.918		56.0	LOS E	55.9	401.5				
North: Dou	ubell Blv	d (Sch	nool Acc	ess)											
Lane 1	170	0.0	170	0.0	300 ¹	0.566	100	59.8	LOS E	9.1	63.8	Full	500	0.0	0.0
Lane 2	108	0.0	108	0.0	164	0.660	100	70.4	LOS E	5.6	39.2	Short	25	0.0	NA
Approach	278	0.0	278	0.0		0.660		63.9	LOS E	9.1	63.8				
West: Say	ers Roa	d													
Lane 1	26	0.0	26	0.0	1173	0.022	100	22.8	LOS C	0.6	3.9	Short	35	0.0	NA
Lane 2	510	2.0	510	2.0	1057 ¹	0.483	100	25.2	LOS C	17.7	126.4	Full	500	0.0	0.0
Lane 3	114	0.0	114	0.0	184	0.620	100	46.1	LOS D	4.3	30.4	Short	50	0.0	NA
Approach	650	1.6	650	1.6		0.620		28.8	LOSC	17.7	126.4				
All Vehicles	2170	1.8	2170	1.8		0.918		49.6	LOS D	55.9	401.5				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Intersection and Approach LOS values are based on average delay for all lanes.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

1 Reduced capacity due to a short lane effect. Short lane queues may extend into the full-length lanes. Delay and stops experienced by drivers upstream of short lane entry have been accounted for.

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PHASING SUMMARY

Site: 105 [380 Sayers Rd Access - Future PM Peak (Site

Folder: Future Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

380 Sayers Road Access Site Category: Existing Design

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 120 seconds (Site User-Given Cycle Time)

Variable Sequence Analysis applied. The results are given for the selected output sequence.

Timings based on settings in the Site Phasing & Timing dialog

Phase Times determined by the program Phase Sequence: Leading Right Turn

Input Phase Sequence: A, B, B2*, B3*, C, D, D2*, D3*

Output Phase Sequence: A, B, C, D

Reference Phase: Phase A

(* Variable Phase)

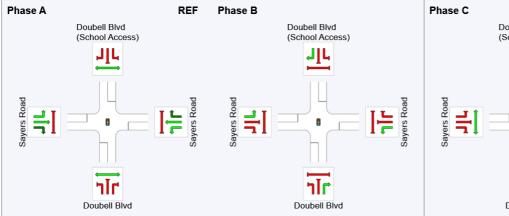
Phase Timing Summary

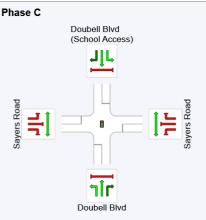
Phase	Α	В	С	D
Phase Change Time (sec)	0	73	85	108
Green Time (sec)	67	6	17	6
Phase Time (sec)	73	12	23	12
Phase Split	61%	10%	19%	10%
Phase Frequency (%)	100.0 ⁴	100.0 ⁴	100.0 ⁴	100.0 ⁴

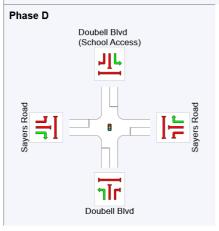
See the Timing Analysis report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Minor Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.

4 Phase Frequency specified by the user (phase times not specified).

Output Phase Sequence









REF: Reference Phase VAR: Variable Phase



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ADVERTISED PLAN

V Site: 104 [300 Sayers Rd Access - Future AM Peak (Site

Folder: Future Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

300 Sayers Road Access Site Category: Existing Design Give-Way (Two-Way)

	Dem Flo	ws	Arrival		Сар.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% B Que	eue	Lane Config	Lane Length	Cap. F Adj. B	
	[Total veh/h	HV] %	[Total veh/h	HV] %	veh/h	v/c	%	sec		[Veh	Dist] m		m	%	%
East: Say	ers Road	b													
Lane 1	1056	3.2	1056	3.2	1930	0.547	100	0.1	LOS A	0.0	0.0	Full	180	0.0	0.0
Lane 2	64	0.0	64	0.0	588	0.109	100	10.4	LOS B	0.4	2.7	Short	35	0.0	NA
Approach	1120	3.0	1120	3.0		0.547		0.7	NA	0.4	2.7				
West: Say	ers Roa	d													
Lane 1	20	0.0	20	0.0	1828	0.011	100	5.6	LOS A	0.0	0.0	Short	35	0.0	NA
Lane 2	700	3.9	700	3.9	1922	0.364	100	0.1	LOS A	0.0	0.0	Full	415	0.0	0.0
Approach	720	3.8	720	3.8		0.364		0.3	NA	0.0	0.0				
All Vehicles	1840	3.3	1840	3.3		0.547		0.5	NA	0.4	2.7				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: 104 [300 Sayers Rd Access - Future PM Peak (Site

Folder: Future Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

300 Sayers Road Access Site Category: Existing Design Give-Way (Two-Way)

Lane Use	e and F	erfor	mance												
	Dem Flo [Total	WS	Arrival [Total		Сар.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Ba Que [Veh		Lane Config	Lane Length	Cap. F Adj. B	
	veh/h	%	veh/h	%	veh/h	v/c	%	sec		[VOII	m m		m	%	%
East: Say	ers Roa	d													
Lane 1	688	2.1	688	2.1	1944	0.354	100	0.0	LOS A	0.0	0.0	Full	180	0.0	0.0
Lane 2	6	0.0	6	0.0	572	0.010	100	10.1	LOS B	0.0	0.3	Short	35	0.0	NA
Approach	694	2.1	694	2.1		0.354		0.1	NA	0.0	0.3				
West: Say	ers Roa	ıd													
Lane 1	10	0.0	10	0.0	1828	0.005	100	5.6	LOS A	0.0	0.0	Short	35	0.0	NA
Lane 2	730	1.4	730	1.4	1953	0.374	100	0.1	LOSA	0.0	0.0	Full	415	0.0	0.0
Approach	740	1.4	740	1.4		0.374		0.2	NA	0.0	0.0				
All Vehicles	1434	1.7	1434	1.7		0.374		0.2	NA	0.0	0.3				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: 103 [Marquands Rd Gate A - Future AM Peak (Site

Folder: Future Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Gate A Access

Site Category: Existing Design

Give-Way (Two-Way)

Lane Use	e and P	erfori	mance												
	Dem Flo	ws	Arrival		Сар.	Deg. Satn	Lane Util.		Level of Service	95% B	eue	Lane Config	Lane Length	Cap. P Adj. B	
	[Total veh/h	HV] %	[Total veh/h	HV] %	veh/h	v/c	%	sec		[Veh	Dist] m		m	%	%
South: Ma	ırquands	Road													
Lane 1	504	2.9	504	2.9	1934	0.261	100	0.1	LOS A	0.0	0.0	Full	500	0.0	0.0
Approach	504	2.9	504	2.9		0.261		0.1	NA	0.0	0.0				
North: Ma	rquands	Road													
Lane 1	382	7.1	382	7.1	1883	0.203	100	0.1	LOSA	0.0	0.0	Full	500	0.0	0.0
Approach	382	7.1	382	7.1		0.203		0.1	NA	0.0	0.0				
West: Sch	ool Acce	ess Rd													
Lane 1	6	0.0	6	0.0	984	0.006	100	1.9	LOS A	0.0	0.2	Full	500	0.0	0.0
Lane 2	16	0.0	16	0.0	491	0.033	100	6.2	LOSA	0.1	0.7	Full	500	0.0	0.0
Approach	22	0.0	22	0.0		0.033		5.0	LOS A	0.1	0.7				
All Vehicles	908	4.6	908	4.6		0.261		0.2	NA	0.1	0.7				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: 103 [Marquands Rd Gate A - Future PM Peak (Site

Folder: Future Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Gate A Access

Site Category: Existing Design

Give-Way (Two-Way)

Lane Use	e and P	erfori	mance												
	Dem Flo	WS	Arrival		Сар.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Ba Que	eue	Lane Config	Lane Length	Cap. P Adj. B	
	[Total veh/h	HV] %	[Total veh/h	HV] %	veh/h	v/c	%	sec		[Veh	Dist] m		m	%	%
South: Ma	ırquands	Road													
Lane 1	320	4.6	320	4.6	1913	0.167	100	0.0	LOS A	0.0	0.0	Full	500	0.0	0.0
Approach	320	4.6	320	4.6		0.167		0.0	NA	0.0	0.0				
North: Ma	rquands	Road													
Lane 1	372	9.0	372	9.0	1861	0.200	100	0.1	LOS A	0.0	0.0	Full	500	0.0	0.0
Approach	372	9.0	372	9.0		0.200		0.1	NA	0.0	0.0				
West: Sch	ool Acce	ess Rd													
Lane 1	10	0.0	10	0.0	1212	0.008	100	1.0	LOS A	0.0	0.2	Full	500	0.0	0.0
Lane 2	26	0.0	26	0.0	631	0.041	100	4.5	LOSA	0.1	0.9	Full	500	0.0	0.0
Approach	36	0.0	36	0.0		0.041		3.5	LOSA	0.1	0.9				
All Vehicles	728	6.6	728	6.6		0.200		0.2	NA	0.1	0.9				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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ADVERTISED PLAN

∇ Site: 102 [Marquands Rd Gate C - Future AM Peak (Site)

Folder: Future Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Gate D Access

Site Category: Existing Design

Give-Way (Two-Way)

Lane Use	and F	erfori	mance												
	Dem Flo	WS	Arrival		Сар.	Deg. Satn	Lane Util.		Level of Service	95% B Que	eue	Lane Config	Lane Length	Cap. P Adj. B	
	[Total veh/h	HV] %	[Total veh/h	HV J %	veh/h	v/c	%	sec		[Veh	Dist] m		m	%	%
South: Ma	rquands	s Road													
Lane 1	530	2.7	530	2.7	1544	0.343	100	3.4	LOS A	2.0	14.1	Full	500	0.0	0.0
Approach	530	2.7	530	2.7		0.343		3.4	NA	2.0	14.1				
North: Mai	rquands	Road													
Lane 1	438	5.1	438	5.1	1347	0.325	100	4.7	LOS A	1.8	12.9	Full	500	0.0	0.0
Approach	438	5.1	438	5.1		0.325		4.7	NA	1.8	12.9				
West: Willi	iamstow	n Drive	е												
Lane 1	8	25.0	8	25.0	1076	0.007	100	1.2	LOS A	0.0	0.2	Short	15	0.0	NA
Lane 2	24	83.3	24	83.3	226	0.106	100	13.9	LOS B	0.3	3.7	Full	500	0.0	0.0
Approach	32	68.8	32	68.8		0.106		10.7	LOS B	0.3	3.7				
All Vehicles	1000	5.9	1000	5.9		0.343		4.2	NA	2.0	14.1				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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ADVERTISED PLAN

V Site: 102 [Marquands Rd Gate C - Future PM Peak (Site

Folder: Future Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Gate C Access

Site Category: Existing Design

Give-Way (Two-Way)

Lane Use	e and P	erfor	mance												
	Dem Flo	WS	Arrival		Сар.	Deg. Satn	Lane Util.	Aver. Delay	Level of Service	95% Ba Que	eue	Lane Config	Lane Length	Cap. F Adj. E	
	[Total veh/h	HV J %	[Total veh/h	HV J %	veh/h	v/c	%	sec		[Veh	Dist] m		m	%	%
South: Ma	rquands	Road													
Lane 1	322	4.5	322	4.5	336	0.958	100	5.9	LOSA	8.8	64.2	Full	500	-80.0	0.0
Approach	322	4.5	322	4.5		0.958		5.9	NA	8.8	64.2				
North: Ma	rquands	Road													
Lane 1	302	5.1	302	5.1	308	0.979	100	6.3	LOSA	2.6	18.8	Full	500	-82.0	0.0
Approach	302	5.1	302	5.1		0.979		6.3	NA	2.6	18.8				
West: Will	iamstow	n Driv	е												
Lane 1	12	16.7	12	16.7	1138	0.011	100	1.0	LOSA	0.0	0.3	Short	15	0.0	NA
Lane 2	62	51.6	62	51.6	443	0.140	100	6.2	LOS A	0.4	4.4	Full	500	0.0	0.0
Approach	74	45.9	74	45.9		0.140		5.3	LOSA	0.4	4.4				
All Vehicles	698	9.2	698	9.2		0.979		6.0	NA	8.8	64.2				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: 101 [Marquands Rd Gate D - Future AM Peak (Site

Folder: Future Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Gate D Access

Site Category: Existing Design

Give-Way (Two-Way)

Lane Use	and P	erfori	mance												
	Dem Flo ^v [Total veh/h	WS	Arrival [Total veh/h		Cap.	Deg. Satn v/c	Lane Util. %	Aver. Delay sec	Level of Service	95% Ba Que [Veh		Lane Config	Lane Length m	Cap. F Adj. B %	
South: Ma	rquands	Road													
Lane 1	228	5.2	228	5.2	1883	0.121	100	0.2	LOSA	0.0	0.3	Full	500	0.0	0.0
Approach	228	5.2	228	5.2		0.121		0.2	NA	0.0	0.3				
East: Seaf	ord Circ	uit													
Lane 1	16	0.0	16	0.0	329	0.049	100	13.5	LOS B	0.1	1.0	Full	500	0.0	0.0
Approach	16	0.0	16	0.0		0.049		13.5	LOS B	0.1	1.0				
North: Mar	quands	Road													
Lane 1	428	5.4	428	5.4	1897	0.226	100	0.4	LOSA	0.0	0.0	Full	500	0.0	0.0
Approach	428	5.4	428	5.4		0.226		0.4	NA	0.0	0.0				
West: Sch	ool Acce	ess Rd													
Lane 1	436	0.0	436	0.0	1337	0.326	100	1.0	LOS A	1.6	11.3	Full	500	0.0	0.0
Approach	436	0.0	436	0.0		0.326		1.0	LOSA	1.6	11.3				
All Vehicles	1108	3.2	1108	3.2		0.326		0.8	NA	1.6	11.3				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: 101 [Marquands Rd Gate D - Future PM Peak (Site

Folder: Future Conditions)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Gate D Access

Site Category: Existing Design

Give-Way (Two-Way)

Lane Use	and P	erfori	mance												
	Dem Flo ^v [Total veh/h	WS	Arrival [Total veh/h		Cap.	Deg. Satn v/c	Lane Util. %	Aver. Delay sec	Level of Service	95% Ba Que [Veh		Lane Config	Lane Length m	Cap. F Adj. E %	
South: Ma	rquands	Road													
Lane 1	224	5.3	224	5.3	949	0.236	100	0.1	LOSA	0.0	0.1	Full	500	-50.0	0.0
Approach	224	5.3	224	5.3		0.236		0.1	NA	0.0	0.1				
East: Seaf	ord Circ	uit													
Lane 1	12	0.0	12	0.0	487	0.025	100	9.8	LOSA	0.1	0.6	Full	500	0.0	0.0
Approach	12	0.0	12	0.0		0.025		9.8	LOS A	0.1	0.6				
North: Mar	quands	Road													
Lane 1	298	5.3	298	5.3	948	0.314	100	0.6	LOSA	0.0	0.0	Full	500	-50.0	0.0
Approach	298	5.3	298	5.3		0.314		0.6	NA	0.0	0.0				
West: Sch	ool Acce	ess Rd													
Lane 1	416	0.0	416	0.0	1339	0.311	100	1.0	LOSA	1.5	10.6	Full	500	0.0	0.0
Approach	416	0.0	416	0.0		0.311		1.0	LOSA	1.5	10.6				
All Vehicles	950	2.9	950	2.9		0.314		0.8	NA	1.5	10.6				

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Lane LOS values are based on average delay per lane.

Minor Road Approach LOS values are based on average delay for all lanes.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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