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Tragowel BESS, Kerang HYDROLOGY AND STORMWATER MANAGEMENT TRANSMISSION CONNECTION ASSESSMENT

April 2026

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Executive Summary

This report presents the hydrology, flooding and stormwater management strategy for the proposed Tragowel Battery Energy Storage System (BESS). The proposed utility installation is located at Crown Allotment 3 (PT) Parish of Kerang, Lot 1 on Title Plan 601295P in Tragowel, Victoria.

The primary landholding for the proposed Tragowel BESS encompasses 23 ha, divided into four (4) paddocks. Within this broader landholding, the proposed Tragowel BESS will be situated within the largest paddock, with a total area of 12.3 ha. The Proposed BESS Development Area within the selected paddock will occupy approximately 7 ha. An additional 8 ha of land to the north of the 23-ha paddock, located within the Kerang Solar Plant, has been evaluated for the underground transmission corridor. Together, the 12.3-ha paddock, including the 7-ha Proposed BESS Development Area, and the additional 8 ha allocated for the underground transmission corridor, form the 'Project Site', as referred to throughout this document.

The Project Site is in Tragowel, approximately 7 km south of Kerang and 60 km southeast of Swan Hill. The Kerang Terminal Station is located 2.4 km north-west of the Project Site. The Project Site is located within Gannawarra Shire and the North Central Catchment Management Authority (CMA) region and is proposed to have a BESS utility installation.

The BESS project is proposed to have a 200 MW/800 MWh capacity. The project includes:

- BESS components including batteries, inverters, and transformer units
- Switch rooms and control building
- Transmission connection infrastructure, with underground transmission preferred
- Substation and associated electrical infrastructure
- Operations and maintenance building
- Access points and internal roads
- Water storage systems and fire-fighting infrastructure
- Security fencing and monitoring systems
- Earthworks/benching
- Stormwater detention
- Landscaping/screening.

The Project Site is located within the Loddon River floodplain and is generally flat with an average elevation of 78.3 m AHD. Grades are approximately 0.001 m/m (west to east). The Proposed BESS Development Area naturally grades east to the Piangil Railway Line embankment, and there is no existing outlet from the Proposed BESS Development Area to a waterway. The lack of an existing outlet and the location within a floodplain inform the developed condition stormwater management, as detailed in Section 6.

The investigations undertaken for the proposed utility installation:

- Describe the Proposed BESS Development Area's risk of flooding and include the finished levels of assets set by the CMA.

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- Demonstrate how a cut-fill balance can be achieved. On-site excavation depths will likely be controlled by groundwater depth. Groundwater conditions are discussed in Sections 3.6 and 6.1.6.
- Show how the proposed layout retains overland flow paths and floodplain volumes.
- Discuss impacts to on-site infiltration and surface water quality, including water quality in adjacent land and waterways.
- Include details of how polluted or contaminated runoff can be managed.

In the developed condition:

- The BESS units within the Proposed BESS Development Area will be raised above natural surface levels to protect the utility installation from riverine flooding in accordance with CMA requirements. The required elevation will be provided by a combination of earthworks and an elevated platform (gantry) supported on concrete piles on which the battery units will be located.
- The battery units will be placed at a minimum level of 79.75 m AHD, higher than the CMA's requirement of 79.7 m AHD.
- The substation and control building areas will be filled to a level of 80.62 m AHD, higher than the CMA's requirement of 79.7 m AHD.
- A vegetated swale will be constructed along the southern edge of the Proposed BESS Development Area. The swale will:
 - o Provide overland flow path for runoff from the BESS in major events.
 - o Provide floodplain storage to compensate for a portion of the Proposed BESS Development Area being filled.
 - o Direct stormwater from the Proposed BESS Development Area to intercept dams and detention basins that provide on-site detention (OSD).
 - o Provide a source of fill material.
 - o Provide stormwater quality treatment in excess of statutory requirements.
 - o Outlet to the north-west via a pumped connection.
- Creation of fill pads and construction of the elevated platform (gantry) will ensure protection of the utility installation from external flows.
- An underground transmission corridor to the Kerang Terminal Station generally follows the alignment of the Piangil Railway Line (Melbourne-Swan Hill). The underground transmission corridor is included in the figures in this report.

Drone footage of the Project Site in the existing condition can be accessed via the following QR Code:



The report demonstrates that the proposed utility installation can be developed with minimal impact on existing hydrological conditions. The conceptual stormwater management plan allows for CFA requirements for containment of firefighting runoff to be achieved. The proposed stormwater management provides green engineering solutions that treat stormwater to improve downstream water quality beyond statutory requirements.

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1. Introduction

This report presents the hydrology, flooding and stormwater management strategy for the proposed Tragowel Battery Energy Storage System (BESS). The proposed utility installation is located at Crown Allotment 3 (PT) Parish of Kerang, Lot 1 on Title Plan 601295P in Tragowel, Victoria.

The primary landholding for the proposed Tragowel BESS encompasses 23 ha, divided into four (4) paddocks. Within this broader landholding, the proposed Tragowel BESS will be situated within the largest paddock, with a total area of 12.3 ha. The Proposed BESS Development Area within the selected paddock will occupy approximately 7 ha. An additional 8 ha of land to the north of the 23-ha paddock, located within the Kerang Solar Plant, has been evaluated for the underground transmission corridor. Together, the 12.3-ha paddock, including the 7-ha proposed BESS development area, and the additional 8 ha allocated for the underground transmission corridor, form the 'Project Site', as referred to throughout this document.

The Project Site is in Tragowel, approximately 7 km south of Kerang and 60 km southeast of Swan Hill. The Kerang Terminal Station is located 2.4 km north-west of the Project Site. The Project Site is located within Gannawarra Shire and the North Central Catchment Management Authority (CMA) region and is proposed to have a BESS utility installation. The BESS project is proposed to have a 200 MW/800 MWh capacity. The project includes:

- BESS components including batteries, inverters, and transformer units
- Switch rooms and control building
- Transmission connection infrastructure, with underground transmission preferred
- Substation and associated electrical infrastructure
- Operations and maintenance building
- Access points and internal roads
- Water storage systems and fire-fighting infrastructure
- Security fencing and monitoring systems
- Earthworks/benching
- Stormwater detention
- Landscaping/screening.

This report comprises a hydrology investigation and stormwater management strategy (SWMS) for the proposed utility installation. This report:

- Describes the Proposed BESS Development Area's risk of flooding and includes the finished levels of assets set by the CMA.
- Demonstrates how a cut-fill balance can be achieved. Excavation depths within the Proposed BESS Development Area will likely be controlled by groundwater depth. Groundwater conditions are discussed in Sections 3.6 and 6.1.6.
- Discusses how groundwater considerations have informed the location of the underground transmission corridor connecting the Proposed BESS Development Area to the Kerang Terminal Station.
- Shows how the proposed layout retains overland flow paths and floodplain volumes.

- Discusses impacts to infiltration and surface water quality within the Proposed BESS Development Area, including water quality in adjacent land and waterways.
- Includes details of how polluted or contaminated runoff can be managed.

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2. Project Site Overview

2.1 Location

The proposed utility installation is located at Crown Allotment 3 (PT) Parish of Kerang, Lot 1 on Title Plan 601295P in Tragowel, Victoria. The primary landholding for the proposed Tragowel BESS encompasses 23 ha, divided into four (4) paddocks. Within this broader landholding, the proposed Tragowel BESS will be situated within the largest paddock, with a total area of 12.3 ha. The Proposed BESS Development Area within the selected paddock will occupy approximately 7 ha. An additional 8 ha of land to the north of the 23-ha paddock, located within the Kerang Solar Plant, has been evaluated for the underground transmission corridor. Together, the 12.3-ha paddock, including the 7 ha of the Proposed BESS Development Area, and the additional 8 ha allocated for the underground transmission corridor, form the 'Project Site', as referred to throughout this document.

The Project Site is in Tragowel, approximately 7 km south of Kerang and 60 km southeast of Swan Hill. The Kerang Terminal Station is located 2.4 km north-west of the Proposed BESS Development Area. The Project Site is located within Gannawarra Shire and the North Central Catchment Management Authority (CMA) region. As the Project Site is located within Gannawarra Shire, the Infrastructure Design Manual (IDM) is the relevant guiding document for flow calculations and concept drainage design. The North Central CMA is the responsible authority for setting flood levels and approving major event stormwater management.

The Project Site is located within the proposed Northwest Renewable Energy Zone (REZ). The Northwest REZ is designated under the Draft 2025 Victorian Transmission Plan. The plan is currently under consultation. As the REZ development plan progresses and attracts further investment, there will be significant growth in renewable energy generation, transmission, and storage throughout the Northwest region. An overview of the investigation area is shown in Figure 1.

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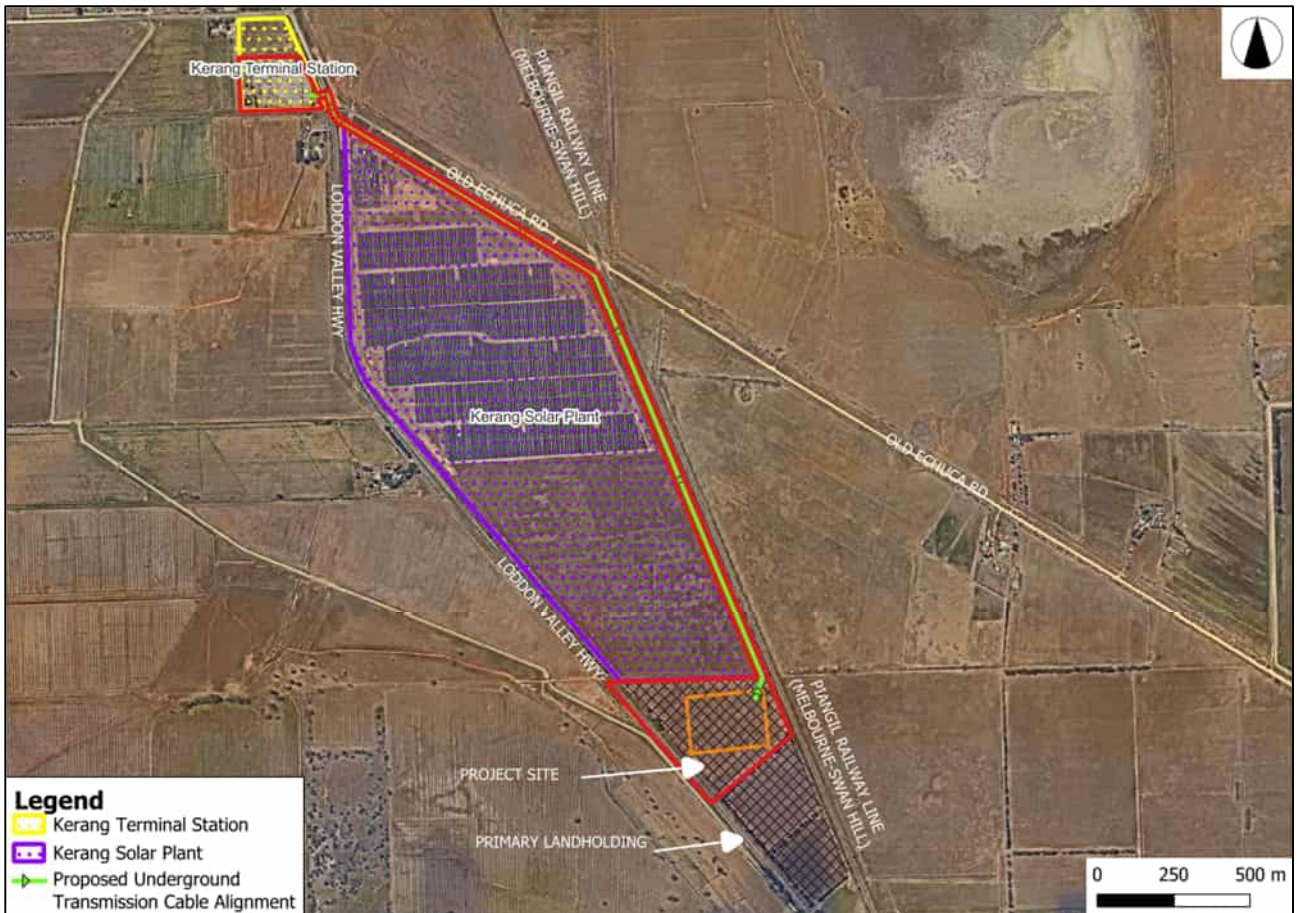


Figure 1: Project Site Overview

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2.2 Existing Land Use

Currently, the Proposed BESS Development Area is rural and is zoned as Farming Zone (FZ). The Proposed BESS Development Area is currently used for grazing sheep and the production of fodder, hay and silage. The Piangil Railway Line (Melbourne-Swan Hill) is adjacent to the eastern boundary. Figure 1 shows the Project Site and Figure 2 shows the Proposed BESS Development Area in the existing condition.

Clause 35.07-4 (Schedule) FZ notes that any earthworks, which change the rate of flow or the discharge of water across a property boundary, or which increase the discharge of saline ground water trigger a permit condition. In this instance, the Proposed BESS Development Area has no outlet or connection to a waterway, as detailed in Section 2.4. The earthworks will change the rate of flow and discharge of water across the property boundary to the north. The stormwater management strategy for the proposed utility installation presented in Section 6 has been developed to minimise the change in flows resulting from the project and ensure that changes to flows are not significant and are effectively managed. If stormwater reuse is possible, as mentioned in Section 6.1.2, further mitigation of changes in the flow regime as a result of the development can be achieved.

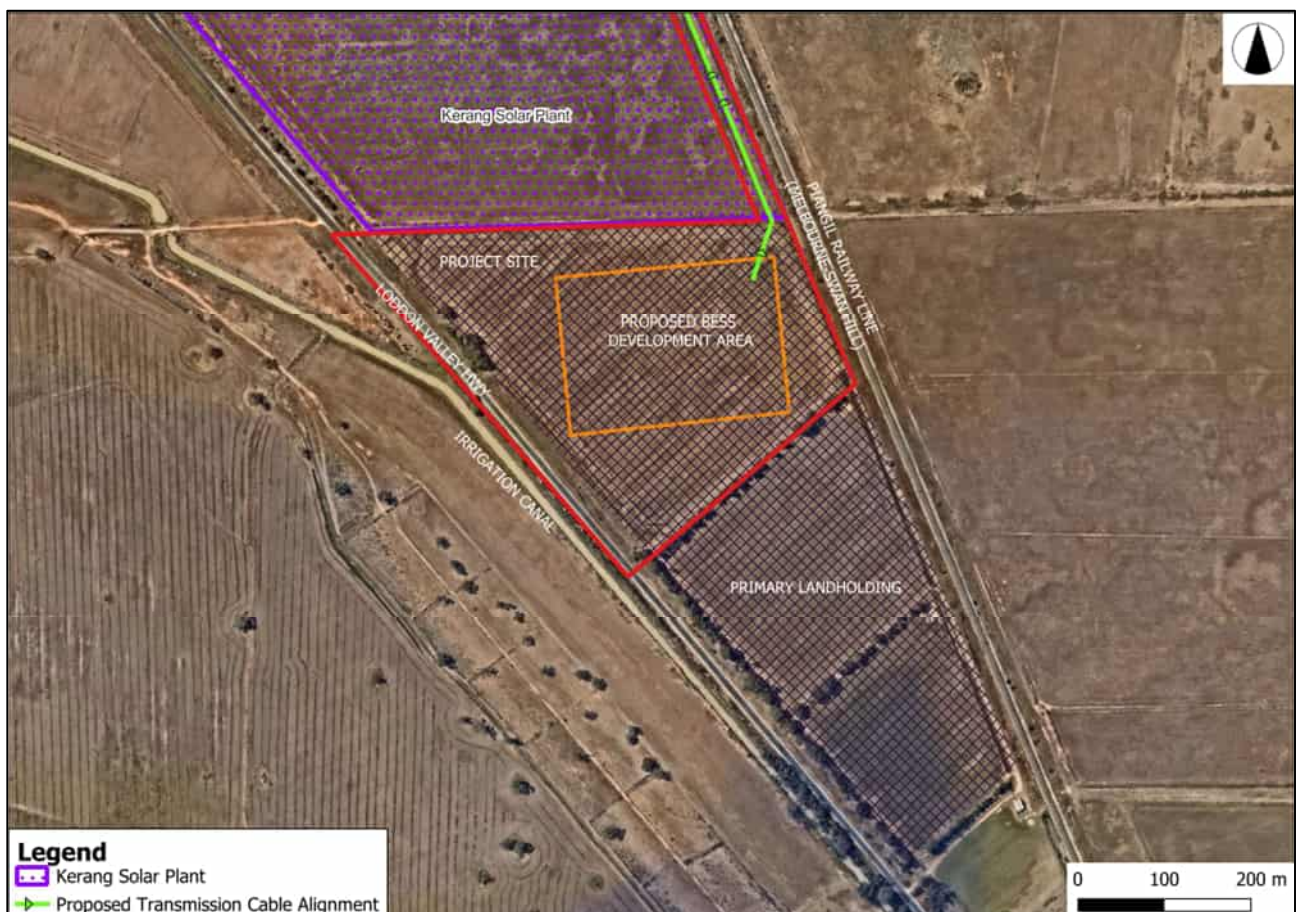


Figure 2: Proposed BESS Development Area in Existing Condition

2.3 Existing Planning Overlays

The primary landholding has two overlays in the existing condition. The siting of the utility installation has been undertaken to minimise impacts and respect the existing planning overlays.

An Environmental Significance Overlay (ESO4) is in place across the entire Project Site (refer to Figure 3). The ESO states flooding is a major issue at the proposed location of the utility installation and lists permit requirements to manage flood storage and flood protection for future development. The requirements of the North Central CMA for maintaining flood storage by balancing cut and fill and for providing flood protection through the finished level of assets have been incorporated into the proposed development proposal, see Section 4. CMA advice is included as Appendix A.

A Specific Controls Overlay (SCO2) is in place across the entire Project Site (refer to Figure 3). This overlay marks the area as part of the Goulburn-Murray Water Connections Project. The SCO2 does not impact hydrology.

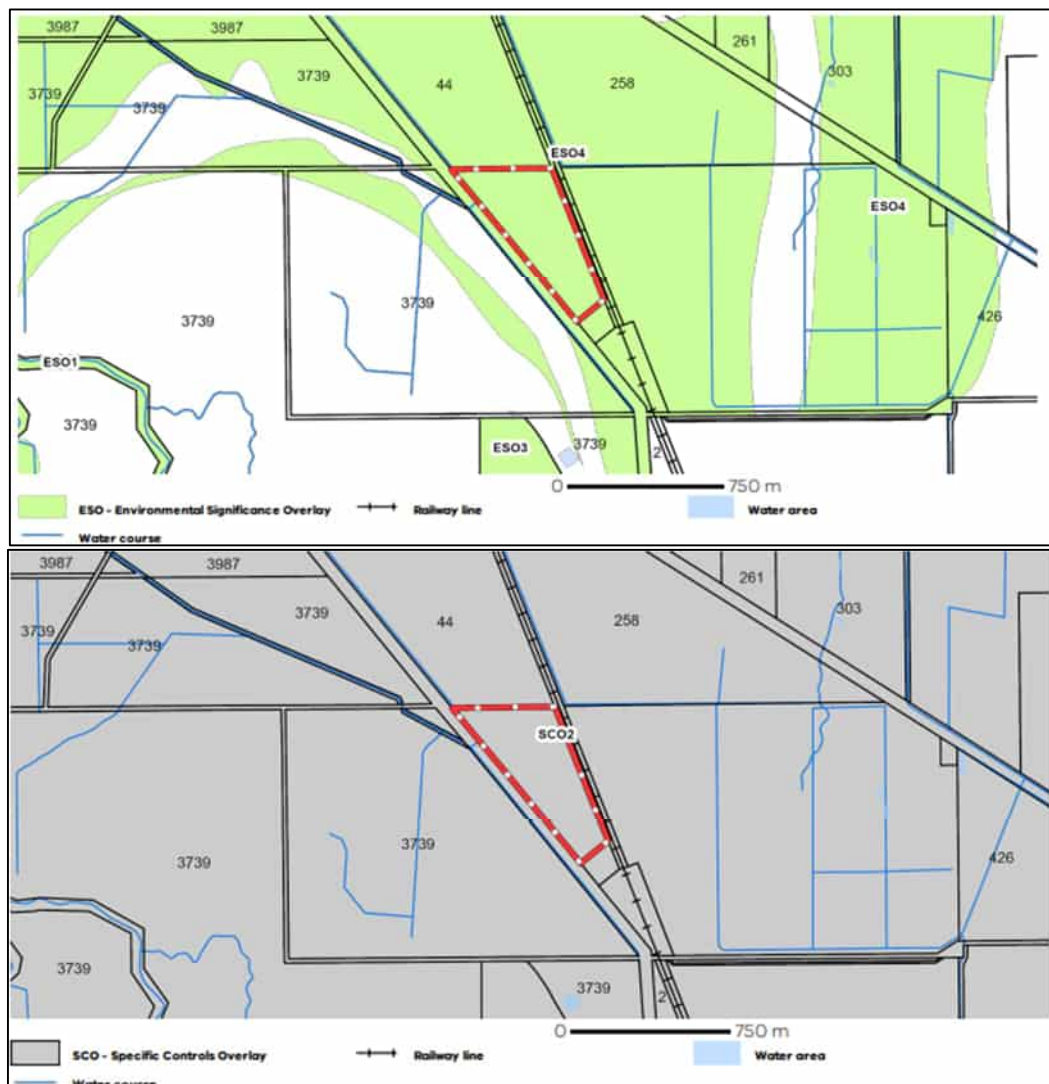


Figure 3: Overlays and Primary Landholding

2.4 Existing Topography

In the existing condition, the Proposed BESS Development Area is generally flat with an average elevation of 78.3 m AHD. Grades across the Proposed BESS Development Area are approximately 0.001 m/m (west to east). The Proposed BESS Development Area grades from the south-west to the north-east and is located within the Loddon River floodplain.

The Loddon Valley Highway along the western boundary has a crest elevation of between 78.50 and 79.45 m AHD adjacent to the Project Site, and the Piangil Railway Line (Melbourne-Swan Hill) embankment along the eastern boundary has a crest elevation of 79.15 m AHD.

There is no outlet from the Proposed BESS Development Area to a waterway. The land naturally grades to the east to the railway embankment. Existing agricultural irrigation practices are to flood the paddocks and allow water to soak into the soil. The lack of an existing outlet and the location of the Proposed BESS Development Area within a floodplain inform the developed condition stormwater management. The natural grade of the land directs water to pond at the east, as shown in a historic aerial image from December 2022 included as Figure 4.

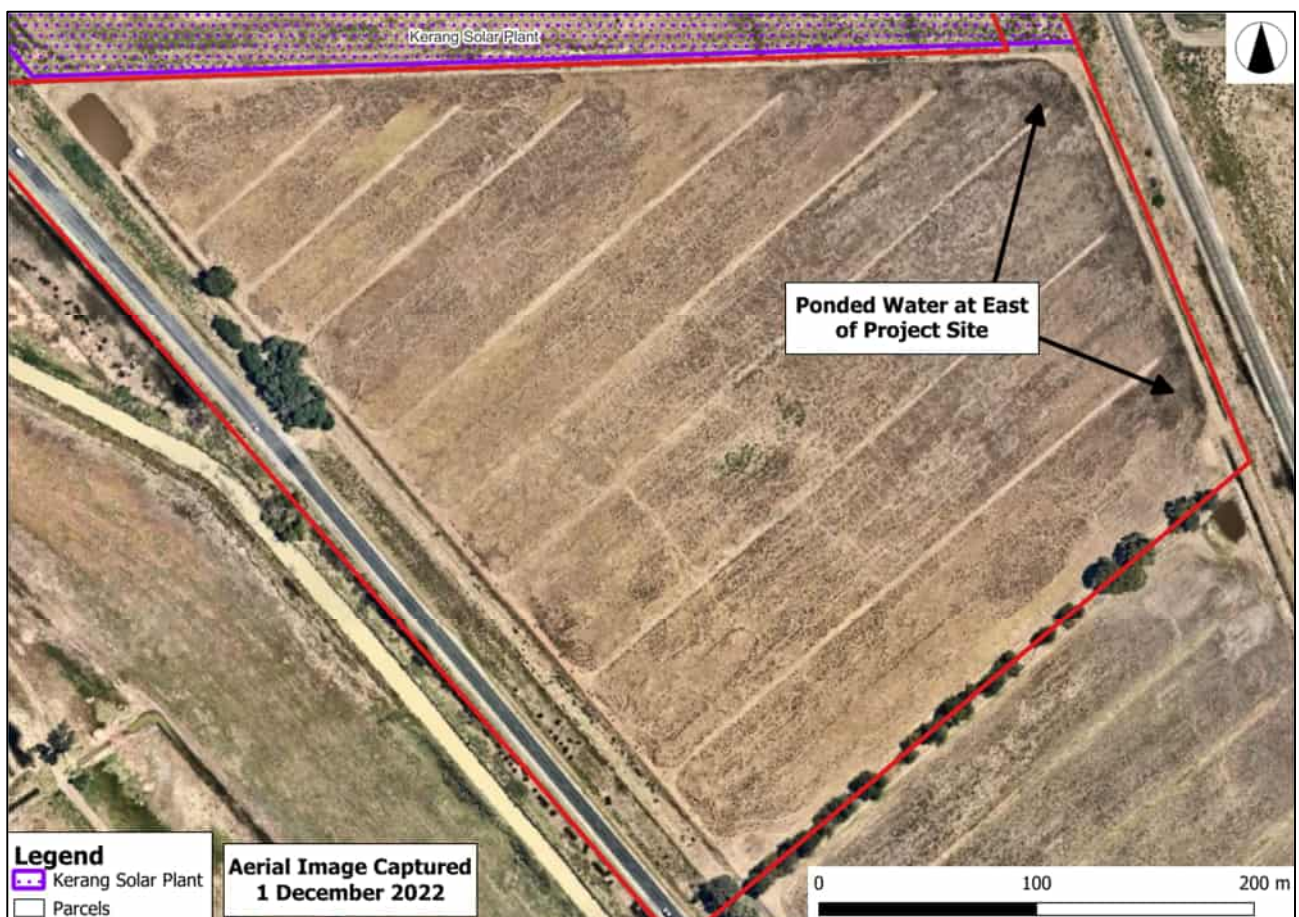
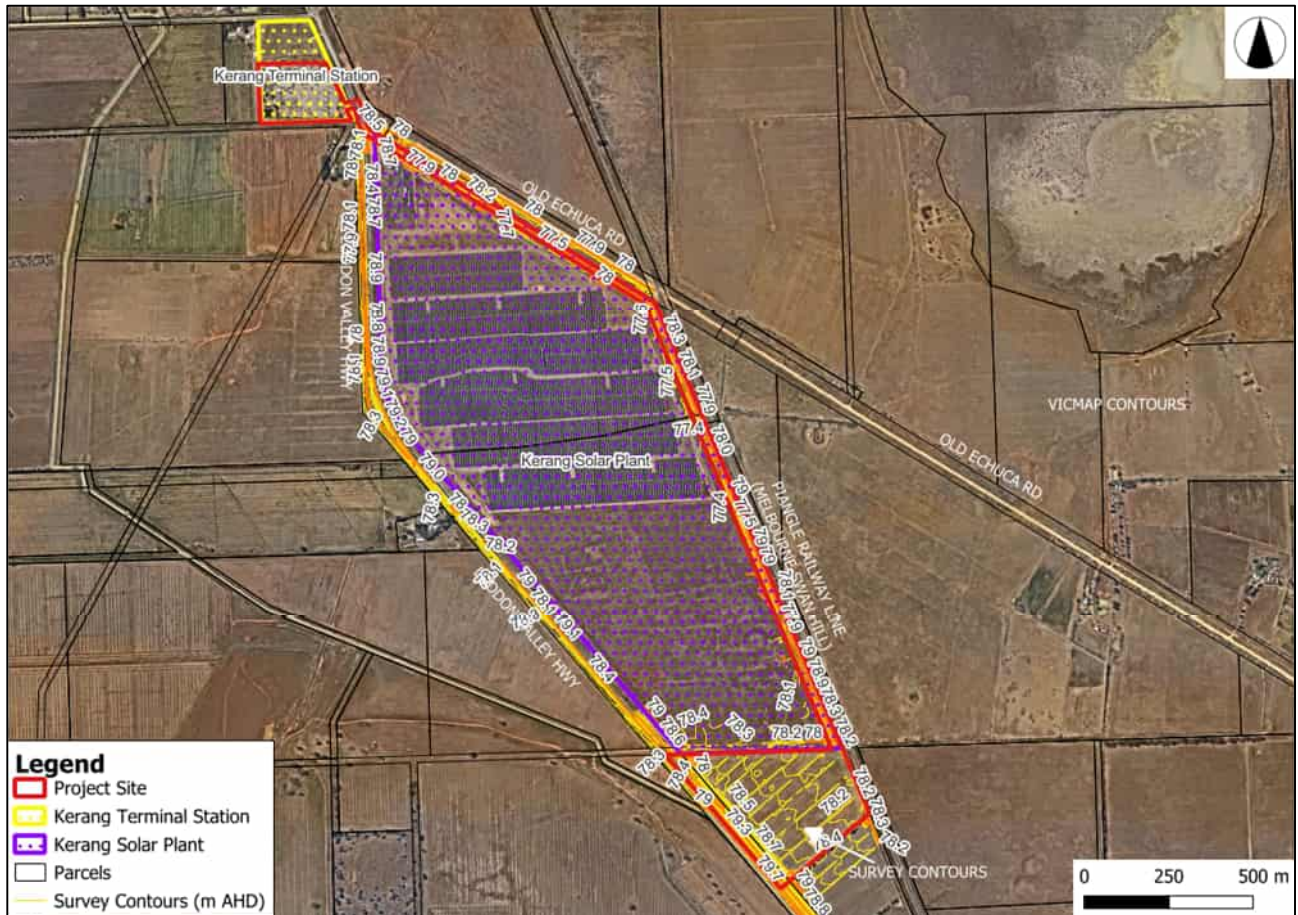


Figure 4: Historic Water Ponding at East of Project Site

As shown in the aerial image, there are existing swales along the western and eastern boundaries of the Project Site. Both swales collect surface water from the south and/or irrigation inflows. Neither

swale provides conveyance beyond the Project Site. Aerial imagery also shows two existing dams at the northwest and southwest. The capacity and function of these dams are currently unknown, although it is likely that they are used for agricultural purposes. Further investigation related to the ecological significance of the dams should be undertaken during detailed design. The contours shown in Figure 5 and Figure 6 are based on Survey data completed by Northern Land Solutions in October 2024. The survey data is included as Appendix B.



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3. Existing Condition Hydrology Investigation

Owing to the flat grades at the Project Site and within the surrounding area of the Loddon River floodplain, the Proposed BESS Development Area is not isolated from external catchments. Discussions with the North Central CMA (See Section 4) indicate that flooding from the Loddon River enters the Proposed BESS Development Area from the west and south. Additional details of existing fluvial (riverine) flooding, including flood levels, are included in Section 4.

Advice received from the North Central CMA and included as Appendix A indicates that fluvial (riverine) flooding enters the Proposed BESS Development Area when the Loddon Valley Highway and adjacent irrigation channel are overtopped.

3.1 Catchments

The Project Site is very flat and is part of the overarching Loddon River system (see Figure 7). The Proposed BESS Development Area drains overland to the north-east via sheet flow, where it ponds without a natural outlet in the existing condition. Small existing farm dams at the Proposed BESS Development Area are believed to supply irrigation water and be filled by pumping (see Figure 8).

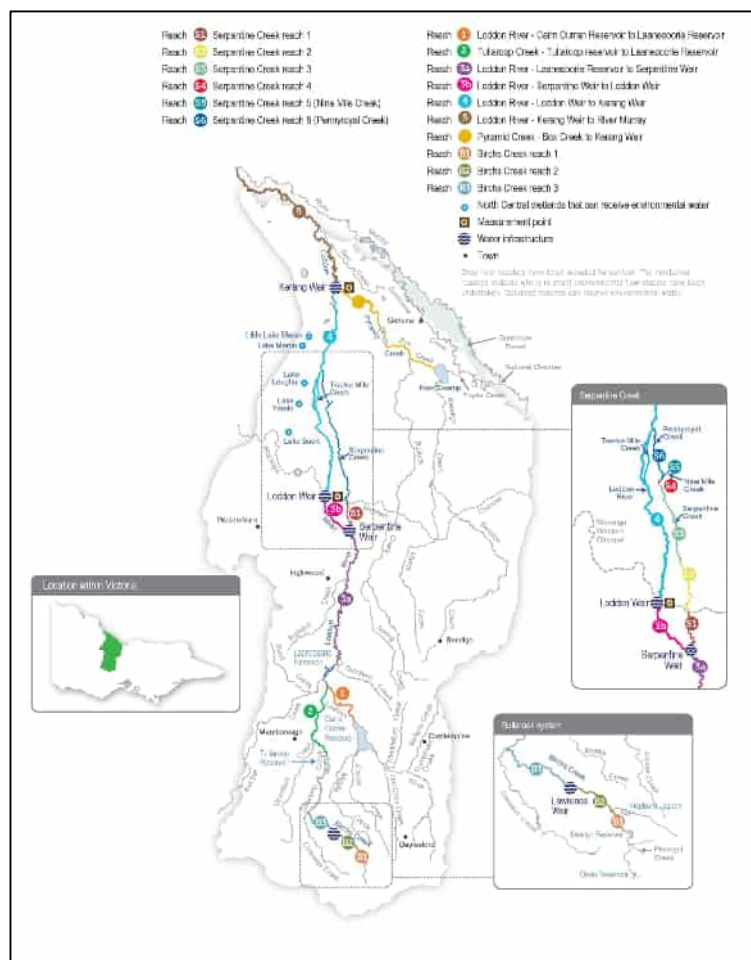


Figure 7: Existing Loddon River System Catchment (Victorian Environmental Holder, 2025)

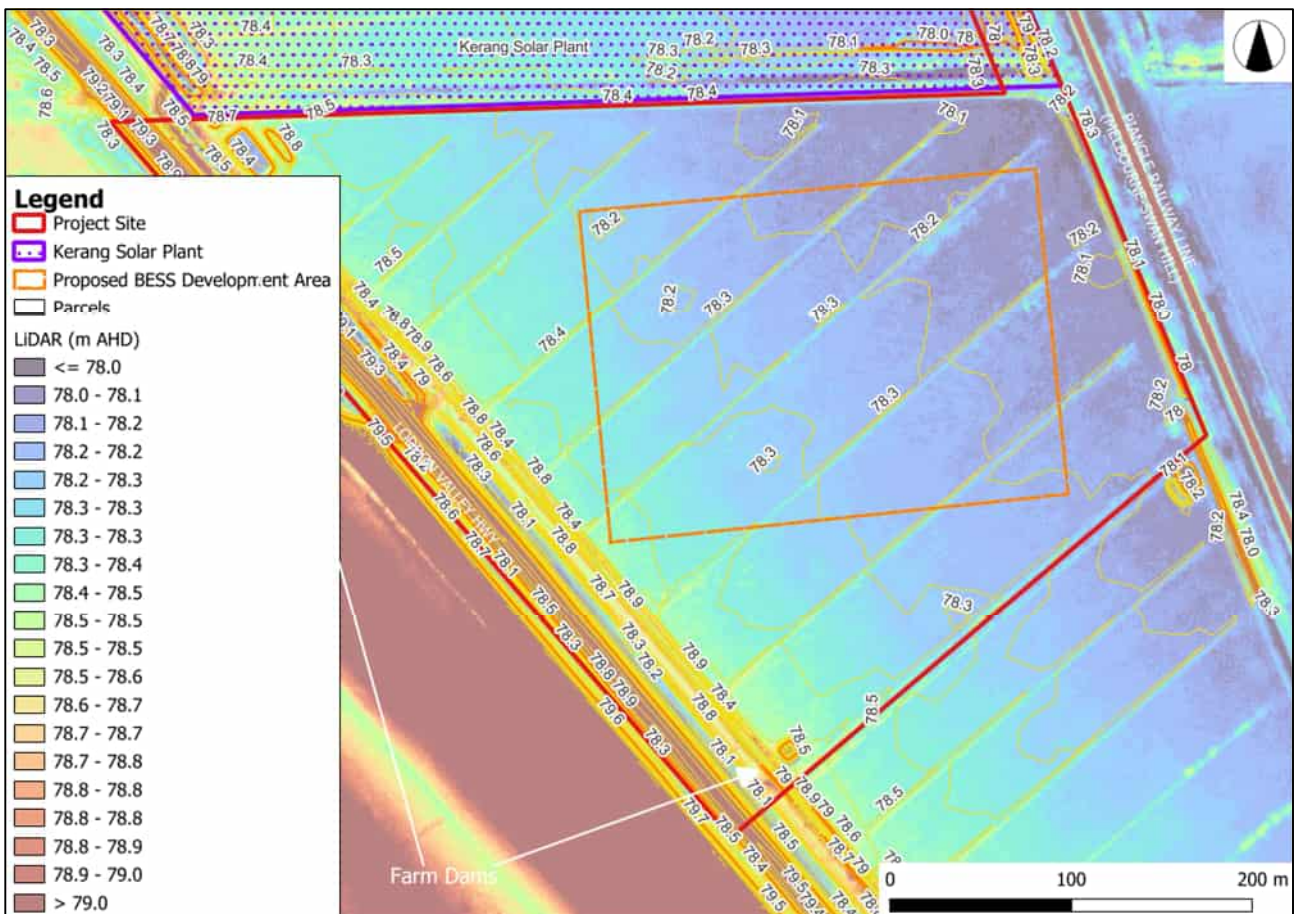


Figure 8: Proposed BESS Development Area Existing Elevations

3.2 Peak Flows

For the purposes of this report, peak minor flows have been defined based on IDM requirements as 10% Annual Exceedance Probability (AEP) flows, and peak major flows have been defined as 1% AEP flows. The peak flow calculations include the climate change adjustment from *Australian Rainfall and Runoff 2019 version 4.2* (Engineers Australia, 2024).

3.2.1 Proposed BESS Development Area Peak Flows

Peak flows have been calculated for the 7 ha of the Proposed BESS Development Area. Peak flows have also been calculated for the largest paddock within the primary landholding of 12.3 ha. The Proposed BESS Development Area is within the 12.3 ha of the largest paddock.

The two catchments have been selected as the Proposed BESS Development Area will be where development results in changes to the existing flow regime that must be mitigated, as detailed in Section 6.1.2. The largest paddock has been considered as the berms around its boundary hydraulically isolate it, and the Proposed BESS Development Area within it, in storm events that do not result in the Loddon River flooding. Peak existing flows are shown in Table 1. Complete drainage calculations are included in Appendix C. Both existing catchments included in Table 1 have the same

characteristics in the existing condition. The difference in peak flows is a result in the difference in area between the catchments.

Table 1: Existing Peak Flows

Location	Upstream Catchment Area (ha)	Existing 10% AEP Flow (m ³ /s)	Existing 10% AEP Flow (m ³ /s) Climate change SSP5 to 2060	Existing 1% AEP Flow (m ³ /s)	Existing 1% AEP Flow (m ³ /s) Climate change SSP5 to 2060
Proposed BESS Development Area	7	0.3	0.4	0.4	0.6
Largest paddock within primary parcel (contains Proposed BESS Development Area)	12.3	0.4	0.6	0.5	0.9

3.3 Potential for Fluvial/Riverine Flooding

The Project Site is part of the Loddon River floodplain. Owing to the flat grades at the Proposed BESS Development Area and within the surrounding Loddon River floodplain, the Project Site is not isolated from external catchments. Discussions with the North Central CMA indicate that flooding from the Loddon River enters the Proposed BESS Development Area from the west and south when irrigation infrastructure along the Loddon Valley Highway is overtopped. Additional details of existing fluvial (riverine) flooding, including flood levels, are included in Section 4. Complete CMA advice is included in Appendix A. The CMA have advised that the Project Site flooded during the January 2011 Loddon River flood event.

A discussion of how the proposed development will address the CMA requirements is included in Section 4.

3.4 Overland Flow Paths

Aerial imagery does not identify a single point outlet from the Proposed BESS Development Area. The land naturally slopes down to the east, and a swale, used to convey irrigation water, is located along the eastern edge of the Proposed BESS Development Area that ends at the north-eastern boundary. The railway embankment and end of the swale contain overland flows within the Proposed BESS Development Area.

3.5 Losses and Permeability

The Proposed BESS Development Area is used for sheep grazing. The land is compacted from its agricultural use. While defining permeability is outside the scope of this report, the existing land use within the Project Site does not allow for substantial soil permeability. Rainfall losses are impacted by the permeability of the soil and the existing vegetation.

3.6 Groundwater Assessment

An assessment of existing groundwater conditions was undertaken by SWM Consulting during preparation of the FIA Report (SWM, February 2025). The desktop study utilised publicly available

information provided by the Victorian Department of Energy, Environment, and Climate Action (DEECA). The desktop investigation noted that the closest recorded groundwater depth is at a bore located approximately 1.6 km north-east of the Project Site along Old Echuca Road near Dry Lake. The water level at this gauge was recorded in 2016. The gauge indicated a bore water level of 74.32 m AHD in 2016, suggesting that groundwater is between 2.5 m to 4.0 m below natural surface elevation.

Phil Dyson of the North Central CMA provided a separate assessment of groundwater conditions at the Project Site by email 25 March 2025. The CMA has access to additional groundwater information. The CMA suggested that groundwater could range from 1 to 2 m below natural surface elevation. The email is included with CMA correspondence in Appendix A.

The level of groundwater at the Proposed BESS Development Area is likely to vary locally and seasonally. Further groundwater assessment is outside the scope of the hydrology investigation. It is recommended that a geotechnical engineer be engaged to undertake a detailed groundwater analysis at the Project Site, including the underground transmission corridor, during detailed design to ensure that the designs of underground cabling and concrete foundation piles are appropriate for the groundwater conditions. The depth of groundwater will impact the depth of cut possible in the developed condition. See Section 6.1.6 for additional discussion regarding the underground transmission corridor and the impact of groundwater.

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4. CMA Floodplain Requirements

Communication with the CMA was undertaken by email, online meetings, and phone calls. The CMA provided written advice regarding design requirements within the ESO4 overlay 21 August 2025. CMA requirements are included as Appendix A to this report.

Generally, the CMA require:

- Finished floor levels of all buildings and critical infrastructure to be set no lower than 79.7 m AHD.
- No obstruction of flood flows for reduction in flood storage. Flood conveyance in all directions and a cut-fill balance are required.
- Safe access must be provided, even during periods of prolonged flooding.
- The proposed development must be sited on a fill pad set no lower than 79.4 m AHD. (See Section 4.4)
- The design of the fill pad to be such that it is protected it from wave action erosion during prolonged periods of inundation. (See Section 4.4)
- Preparation of a flood response plan.
- Achievement of best practice stormwater guidelines.

4.1 Design Finished Levels

The CMA have advised that the finished floor level of all buildings and critical infrastructure must be constructed no lower than 79.7 m AHD. As shown in Section 5, the conceptual design of the proposed utility installation provides additional elevation of assets above this level.

4.2 Flood Obstruction

The CMA have advised that flood conveyance must be provided from south to north as well as from east to west. As shown in Section 5, the conceptual design of the development incorporates a raised platform (gantry) and minimises fill to maintain flood conveyance.

The CMA have advised that a floodplain cut-fill balance must be achieved relative to the January 2011 flood level of 78.6 m AHD. As shown in Section 6.2.2, the cut-fill balance can be achieved by the development.

4.3 Safe Access

The CMA have advised that safe vehicular access to the proposed utility installation is required, even during extended inundation events. To provide safe vehicular access, appropriate flood depth and velocities must be maintained on access roads. The detailed design of the access roads will ensure safe access to the Project Site in the developed condition.

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4.4 Fill Pad Requirements

A fill pad is an engineer-designed, constructed platform generally made of compacted soil and/or crushed rock. The purpose of the fill pad requirement by the CMA is to elevate the proposed utility installation above the flood level so that assets, such as batteries, are protected during times when the Proposed BESS Development Area floods.

The CMA have advised that fill pads within the Proposed BESS Development Area should be filled to a minimum level of 79.4 m AHD. Furthermore, the side slopes of the pads should provide erosion protection against wave action likely to occur during extended periods of inundation. The CMA advice has assumed a design for the development where a single fill pad will be created and 300 mm of freeboard to flood levels will be provided above the fill pad by footings or slabs.

The fill pad proposed for the substation area of the Proposed BESS Development Area is proposed to be raised to a level of 80.62 m AHD, as detailed in Section 5. Complete engineering drawings (by others) showing the extent and levels of fill to be placed are included in Appendix D. During detailed design, the sides of the embankment should be designed to withstand wave action in accordance with CMA requirements.

The proposed design for the BESS, see Section 5, has adopted an elevated platform on top of a fill pad within the battery storage area to reduce the amount of fill required. As shown in Section 5, a lower fill pad for the battery storage area is proposed with the required level of assets to be provided by the raised platform (gantry). The design presented in Section 5 incorporates a lower fill pad that does not achieve the CMA fill level requirements. The lower fill pad has been intentionally selected to ensure that the CMA requirements for not obstructing flood flows or reducing flood storage are achieved. The proposed design has been developed to satisfy the CMA's floodplain management intentions.

During detailed design, the sides and top of the fill pad under the battery storage should be designed to withstand wave action and complete inundation so that it is not eroded during times when the Proposed BESS Development Area floods.

4.5 Other Requirements

A flood response plan can be prepared during detailed design. Best practice stormwater guidelines will be achieved through compliance with IDM design standards.

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5. Proposed Project

The conceptual project layout plan (development plan) is shown in Figure 9 and a larger image is included as Appendix D. A stormwater management plan including drainage and stormwater management assets is included as Figure 11. As shown in Figure 9, the Proposed BESS Development Area comprises approximately 7 ha. The proposed development is a utility installation including:

- BESS components including batteries, inverters, and transformer units
- Switch rooms and control building
- Transmission connection infrastructure, with underground transmission preferred
- Substation and associated electrical infrastructure
- Operations and maintenance building
- Access points and internal roads
- Water storage systems and fire-fighting infrastructure
- Security fencing and monitoring systems
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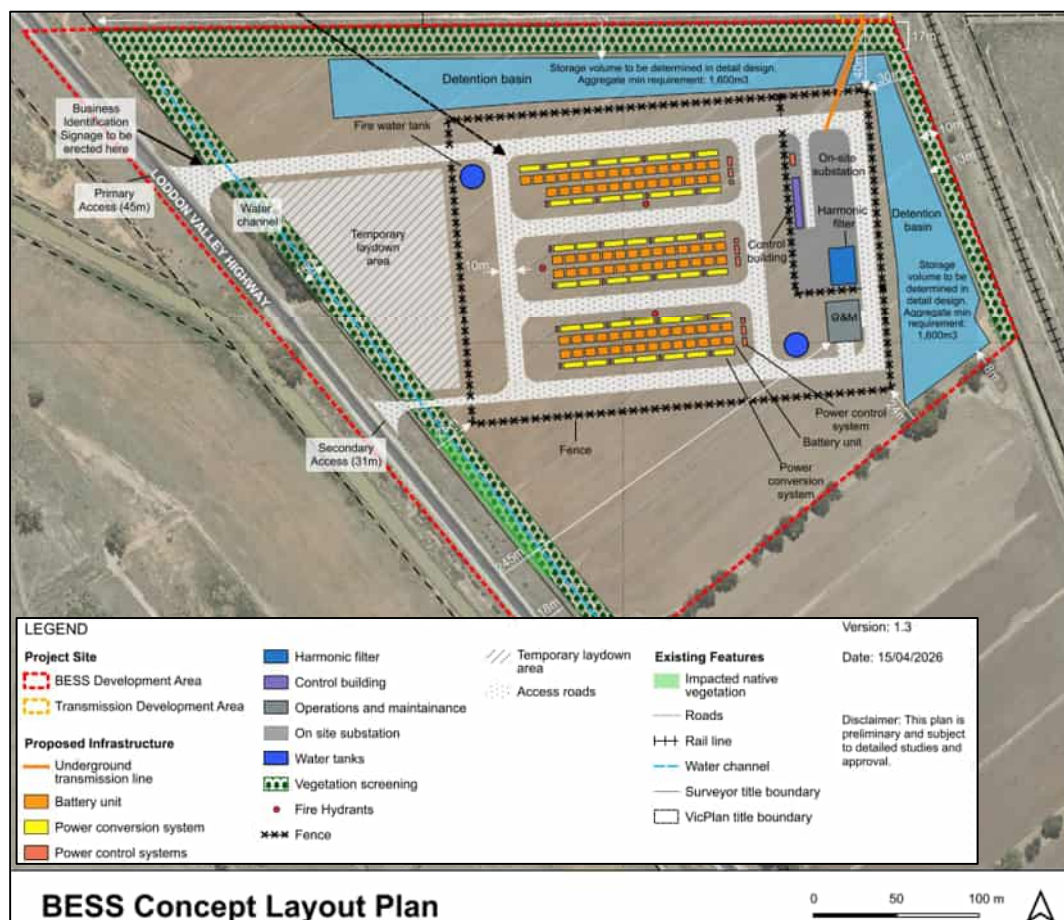


Figure 9: Proposed Project Layout Plan

As shown in Figure 10, the project plan also includes preliminary earthworks design showing finished levels. Preliminary earthworks plans are included in Appendix D.

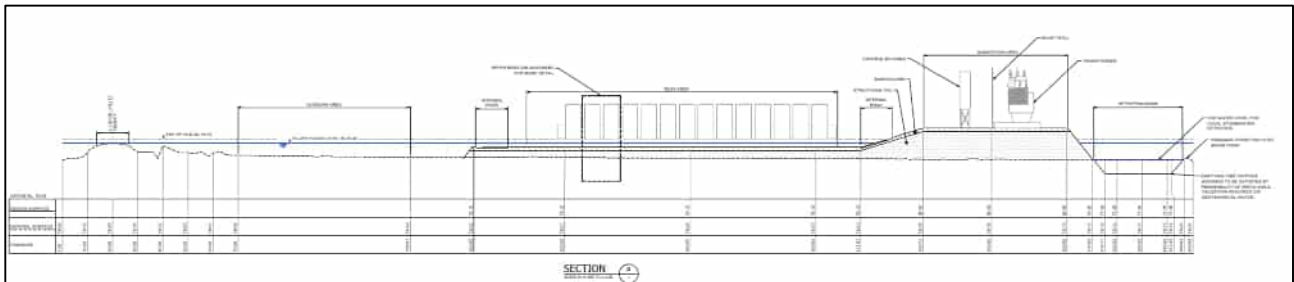


Figure 10: Preliminary Design Cross-section

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6. Stormwater Management

Stormwater can be effectively managed at the Proposed BESS Development Area in the developed condition. Management of stormwater can both protect the installation from major fluvial flooding and ensure that there are minimal changes to existing flow regimes. Opportunities exist for integrated water management and green infrastructure to be incorporated into the design of the proposed utility installation. An overview stormwater management plan for the Proposed BESS Development Area during typical conditions is included in Figure 11. Stormwater management during riverine flooding and inundation is detailed in Section 6.2. The components of stormwater management are discussed in this section of the report.



Figure 11: Stormwater Management Overview Plan – Typical Conditions

6.1 Typical Conditions

Typical conditions refer to times when the Proposed BESS Development Area is not experiencing inundation from fluvial (riverine) flooding of the Loddon River. Stormwater management in typical conditions is focused on directing stormwater away from the proposed assets, reducing peak outflows, and providing stormwater quality treatment.

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6.1.1 Typical Conditions – Grading

The proposed developed condition grading ensures stormwater runoff in typical conditions is directed to detention and treatment. The Proposed BESS Development Area will be graded to fall to the proposed vegetated swale and proposed on-site detention (OSD) basin storages shown in Figure 11. The preliminary earthworks plans included in Figure 10 and Appendix D detail the proposed grading.

6.1.2 Typical Conditions – Detention (Retention) Storages

Two detention basins (storages) are proposed in the developed condition. For stormwater management, it is suggested that the basins provide retention storage and be connected to function as a single retarding basin. However, the basins will fulfill multiple stormwater management purposes, beyond retention of peak flows, as described in 6.1.4 and 6.2.2. The basins are proposed at the north and east of the Proposed BESS Development Area, as shown in Figure 11. The size, dimensions and location of the basins will be finalised during detailed design.

The available storage provided by the detention basins allows them to retain peak stormwater flows. Sizing of the volume required to reduce peak outflows to those experienced in the existing condition for the 1% AEP event has been undertaken using Boyd's Method. Complete drainage calculations are included in Appendix C.

- A storage volume of approximately 1,100 m³ is required to limit 1% AEP outflows from the 7-ha developed area to those from 7 ha of undeveloped area.
- A storage volume of 1,600 m³ is required to limit 1% AEP outflows from the 7-ha developed area to those from 7 ha of undeveloped area when accounting for climate change.

Note that additional storage volume is required to manage the cut-fill balance, as detailed in Section 6.2.2. Therefore, it is possible to provide detention whilst accounting for climate change at no additional cost to the project. Exact detention volume requirements should be confirmed during detailed design.

The Proposed BESS Development Area is not adjacent to or hydraulically connected to any existing waterways. Existing surface contours indicate that it is most appropriate for pumped outflows from the development's storages to be directed north into the Kerang Solar Plant facility. The pumped outlet can be located adjacent to the underground transmission corridor alignment. It is suggested that the design of the pumped outflow incorporate methods to disperse flows so that concentrated outflows from the storage does not contribute to downstream erosion.

The proposed storages provide an opportunity for stormwater reuse, and it is recommended that the project liaise with adjacent landowners to see if it is possible for stormwater to be repurposed for agricultural use. Reuse of stormwater will further minimise the impact of the proposed development.

6.1.3 Typical Conditions – Vegetated Swale

A vegetated swale is proposed to direct runoff from the fill pad within the Proposed BESS Development Area to the proposed detention basins (storages). The swale provides a green engineering solution to reduce stormwater runoff volumes and provides treatment in excess of

regulatory requirements, as detailed in 6.1.5. The swale is included in the preliminary earthworks cross-section shown in Figure 10 and Appendix D.

6.1.4 Typical Conditions – Intercept Dam

The detention basins (storages) also serve an important role in surface water management. As all runoff from the Proposed BESS Development Area is isolated from external flows, water quality from the developed area can be managed. The retention storages allow for stormwater to be disconnected from the surrounding hydrological system by disconnecting the outlet pump.

In case of an environmental incident, the pump from the storages can be stopped—keeping any contaminated runoff on-site and separate to downstream waterways. In this way, the retention storages will function as intercept dams. This achieves Country Fire Authority (CFA) requirements for containing runoff on-site.

6.1.5 Typical Conditions – Stormwater Quality Treatment

Stormwater quality treatment to achieve Best Practice Environmental Management Guidelines (BPEMG) standards is not applicable to the proposal, since the proposed development is a utility installation.

The proposed stormwater management provides an opportunity to integrate stormwater quality treatment into the project at minimal additional cost. The vegetated swale that will collect and convey stormwater from the BESS will be vegetated. The vegetated conveyance swale and passage through the Detention basins (storages) will provide stormwater quality treatment.

MUSIC modelling was undertaken to define conceptually the amount of treatment that can be provided by the swale and detention basins. Rainfall data from the Walpeup Research rain gauge for 2011 to 2021 was adopted. The average annual rainfall in this data set is 350 mm. The MUSIC model schematic is shown in Figure 12. During detailed design, the exact treatment performance of the proposed swale can be assessed once grading and swale dimensions are finalised. Conceptual stormwater quality treatment results are shown in Table 2. If stormwater is able to be reused for agricultural purposes, additional stormwater quality treatment will be provided, and the reuse and subsequent treatment can be modelled in MUSIC to quantify the treatment.

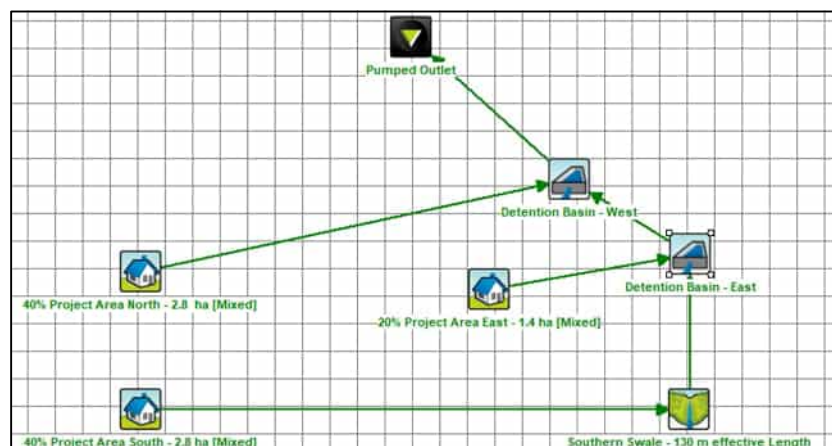


Figure 12: MUSIC Modelling Schematic

Table 2: Stormwater Quality Treatment

Pollutant	BPEM Target reduction %	Pollutant Reduction %
Total Suspended Solids (TSS)	80%	80%
Total Phosphorus (TP)	45%	60%
Total Nitrogen (TN)	45%	30%

6.1.6 Typical Conditions – Infiltration and Groundwater

The majority of the Proposed BESS Development Area's surface is proposed to be crushed rock. A crushed rock surface allows continued permeability. In addition, the proposed use of overland flow paths and a vegetated swale for conveyance of internal stormwater promotes infiltration and minimises impacts of the development on existing groundwater conditions.

As shown in Figure 9 and Figure 11, an underground transmission corridor to the Tragowel Substation is proposed. The alignment of the underground transmission corridor has been based on managing the impacts of groundwater on the design of the asset. The design of the underground transmission corridor will require additional investigations in the detailed design phase because the design will be impacted by groundwater levels. As a generic solution, it is possible to provide the necessary cover requirements to the underground transmission corridor by providing berm protection that allows an underground transmission cable to be laid in the corridor above both flood and water table impacts, as shown diagrammatically in Figure 13. Note that higher levels of groundwater (water table), including the 1-2 m below surface estimated by the CMA, will require further design responses. It is possible that marine cabling may be required to protect assets within the underground transmission corridor from high groundwater levels.

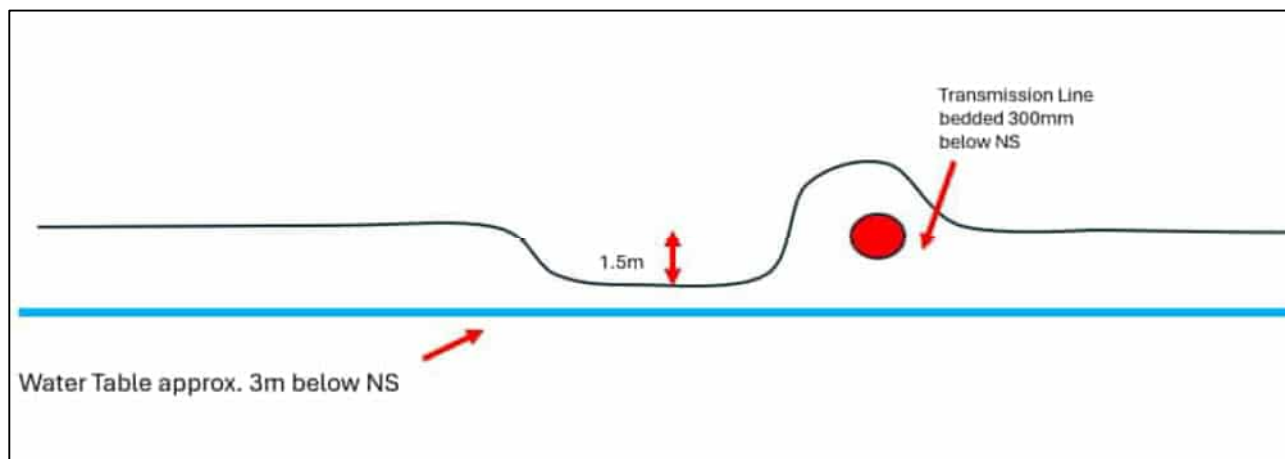


Figure 13: Indicative Underground Transmission Corridor Design Response to Groundwater

It is recommended that a geotechnical professional be engaged to monitor existing groundwater conditions and to provide a detailed assessment of existing and proposed groundwater conditions to inform the detailed design of the proposed development and underground transmission connection.

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6.2 Inundation Conditions

Inundations conditions refer to times when the Proposed BESS Development Area is inundated by external fluvial (riverine) flooding from the Loddon River. In these conditions, the design will achieve the specific floodplain management outcomes required by the North Central CMA.

6.2.1 Inundation Conditions –Protection of Assets

The proposed grading ensures the proposed utility installation is protected from floodwater in inundation conditions. As shown in Figure 10 and Appendix D, the battery storage area and substation area will be elevated to a level of 79.75 m AHD. This is above the 79.7 m AHD required by the North Central CMA.

6.2.2 Inundation Conditions – Cut-Fill Balance

The proposed grading achieves the North Central CMA requirements for balancing floodplain cut-fill in the developed condition. Based on the preliminary earthworks plans included as Appendix D, approximately 15,640 m³ of fill will be placed to the 78.6 m AHD level used by the CMA to assess cut-fill balance. An equal or greater volume of cut must be provided by the development.

The cut to balance the fill will be made in the proposed detention basins. The exact cut volume and cut-fill balance will be prepared during detailed design.

6.2.3 Inundation Conditions – Flood Conveyance

The grading achieves the North Central CMA requirements for providing floodplain conveyance from south to north and west to east. The innovative plan for using elevated platforms (gantries) to raise the battery storages above the relevant flood level minimises fill and allows for flood conveyance.

6.2.4 Inundation Conditions – Erosion Protection

The design of the proposed fill pads will include protection from wave action associated with prolonged inundation in accordance with North Central CMA requirements.

6.3 Peak Flows

Stormwater runoff from the Proposed BESS Development Area will be directed by a vegetated swale to two detention basins that provide retention storage and serve as intercept dams.

Rational method computations have been used to calculate developed condition peak flows. Post-development peak flows are shown in Table 3. Complete Rational Method peak flow calculations for the Proposed BESS Development Area and the largest paddock within the primary parcel that contains it are included in Appendix C.

The peak flows shown in Table 3 will be reduced by the proposed detention basins (storages) so that existing condition peak flows are not exceeded. The maximum outflows shown in Table 3 are outflows from the Proposed BESS Development Area after development.

Table 3: Developed Peak Flows

	Proposed BESS development area	Largest paddock within primary parcel (contains proposed BESS development)
Upstream Area (ha)	7	12.3
Developed 10% AEP Flow (m ³ /s)	0.8	0.9
Developed 10% AEP Flow – Climate Change SSP5 to 2060 (m ³ /s)	1.2	1.3
Maximum Developed 10% AEP Outflow from Detention Basin (m ³ /s)	0.3	0.4
Maximum Developed 10% AEP Outflow from Detention Basin – Climate Change SSP5 to 2060 (m ³ /s)	0.4	0.6
Developed 1% AEP Flow –(m ³ /s)	1.6	1.7
Developed 1% AEP Flow – Climate Change SSP5 to 2060 (m ³ /s)	2.5	2.7
Maximum Developed 1% AEP Outflow from Detention Basin (m ³ /s)	0.4	0.5
Maximum Developed 1% AEP Outflow from Detention Basin – Climate Change SSP5 to 2060 (m ³ /s)	0.6	0.9

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Conclusion

This report presents the investigations related to flooding, hydrology, and stormwater management, for the Tragowel BESS. This report summarises multiple investigations.

The project team liaised with the North Central CMA. The following design considerations are proposed to mitigate flood risk and impact:

- No assets (including batteries) will be located lower than 79.75 m AHD, exceeding CMA requirements.
- The design incorporates a raised platform (gantry) allows for flow conveyance when the Proposed BESS Development Area is inundated.
- The design includes detention storages that serve multiple stormwater management functions including reducing peak outflows to those experienced in the existing condition and achieving the cut-fill balance required by the CMA.

The proposed BESS layout can be accommodated while stormwater management in typical and inundated conditions is achieved.

In the developed condition, the Proposed BESS Development Area will be graded to drain to proposed detention basins that will serve as retarding basins and intercept dams. A pumped overland outlet will discharge to the adjacent property north of the Proposed BESS Development Area. Peak outflows will not exceed those in the existing condition.

External flows that enter the Proposed BESS Development Area from the west of the Loddon Valley Highway during major flood events will be conveyed through the Proposed BESS Development Area. An elevated platform, or gantry, will support the battery storage units and ensure they are protected from floodwater. The Project Site is within the Loddon River floodplain, and external flows will only enter during major flood events.

The proposed grading of the Proposed BESS Development Area and methodology for managing internal and external stormwater has been developed to allow stormwater quality treatment to be provided through green engineering methods. The vegetated swale and proposed retarding basins that serve as intercept dams will manage runoff from the proposed utility installation, but they will also provide an integrated water management benefit as they provide stormwater quality treatment beyond statutory requirements.

This report demonstrates that flood protection can be achieved by the project as assets can be raised above the design flood level. Additionally, stormwater can be managed in the developed condition to achieve Council and CMA standards.

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Appendices

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Appendix A CMA Advice and Requirements

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NCCMA Ref: NCCMA-F-2024-00667
Date: 21 August 2025

Michael Thompson
Julia Baumann
Dalton Consulting Engineers

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Dear Julia,

Floodplain Management Advice

Development Description: Tragowel Battery Energy Storage System (BESS)
Street Address: Loddon Valley Highway, Tragowel Vic 3579
Cadastral Location: Lot 1 TP601295, Parish Of Kerang
Applicant: Julia Baumann

Thank you for your enquiry of 22 May 2025 seeking flood advice for the above property.

Flood Information

Flood levels for the 1% AEP probability (100 year ARI) have not been determined for this area under the *Water Act 1989*. However, information available at North Central CMA indicates that in the event of a 1% AEP flood event it is likely that the entire property will be subject to inundation.

North Central CMA also advises that this property may be afforded protection by an irrigation channel from flood events up to and including the 1% AEP flood. The condition of these works is unknown and there is currently no formal arrangement for the management of these works as mitigation infrastructure.

Development Advice

North Central CMA would require further information before we could provide advice and/or consent for the proposed Battery Energy Storage System. Detailed plans and computations must be provided detailing how the following can be achieved:

1. *The proposed development must be sited on a fill pad constructed no lower than 79.4 metres AHD. This level has been estimated based on a 0.3 metre freeboard above the adjacent railway line. The railway line is expected to act as a hydraulic control for the site in the case that the irrigation channel to the west of the site, which is not designed or managed for a flood mitigation purpose, were to fail. It also provides an allowance for climate change.*
2. *The embankment of the fill pad must be appropriately designed to protect against erosion due to wave action and prolonged inundation which are likely to impact this site.*
3. *The finished floor level of any buildings and any critical infrastructure must be constructed no lower than 79.7 metres AHD.*
4. *Any new development must not obstruct flood flows or reduce flood storage.*

- a. *The development must preserve flood conveyance from the south to north as well as west to east.*
- b. *A cut and fill balance must be achieved for land within the flood prone area of the site. The storage assessment should be based on the estimated January 2011 flood level of 78.6 metres AHD, not the potential flooding originating from the west in the event that the irrigation channel fails or overtops. Any proposed cut must be self-draining so that the area of cut is actively available for storage during a flood event. The area of cut must also be located in areas which are subject to flooding for the full range of flood events expected to affect the site being filled.*
5. *Any new development must achieve best practice stormwater guidelines.*
6. *Any new development must achieve safe access or detail suitable arrangements based on the need for access to the site during prolonged flooding.*
7. *A flood response plan must be prepared to the satisfaction of North Central CMA and the Responsible Authority.*

Please note, this document does not constitute approval or otherwise of any development at this location.

Should you have any queries, please do not hesitate to contact me on **(03) 5440 1896**. To assist the CMA in handling any enquiries please quote **NCCMA-F-2024-00667** in your correspondence.

Yours sincerely,



Nathan Treloar

Waterways and Floodplain Officer

CC: Julia Baumann, Dalton Consulting Engineers
Grant Speldewinde, MAD3 Engineering

Information contained in this correspondence is subject to the definitions and disclaimers below.

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Attached: Definitions and Disclaimers

Definitions and Disclaimers

1. The area referred to in this letter as the 'proposed development location' is the land parcel(s) that, according to the Authority's assessment, represent(s) the location identified by the applicant. The identification of the 'proposed development location' on the Authority's GIS has been done in good faith and in accordance with the information given to the Authority by the applicant(s) and/or local government authority.
2. While every endeavour has been made by the Authority to identify the proposed development location on its GIS using VicMap Parcel and Address data, the Authority accepts no responsibility for or makes no warranty with regard to the accuracy or naming of this proposed development location according to its official land title description.
3. **AEP** as Annual Exceedance Probability – is the likelihood of occurrence of a flood of given size or larger occurring in any one year. AEP is expressed as a percentage (%) risk and may be expressed as the reciprocal of ARI (Average Recurrence Interval).

Please note that the 1% probability flood is not the probable maximum flood (PMF). There is always a possibility that a flood larger in height and extent than the 1% probability flood may occur in the future.

4. **ARI** as Average Recurrence Interval - is the likelihood of occurrence, expressed in terms of the long-term average number of years, between flood events as large as or larger than the design flood event. For example, floods with a discharge as large as or larger than the 100-year ARI flood will occur on average once every 100 years.
5. **AHD** as Australian Height Datum - is the adopted national height datum that generally relates to height above mean sea level. Elevation is in metres.
6. No warranty is made as to the accuracy or liability of any studies, estimates, calculations, opinions, conclusions, recommendations (which may change without notice) or other information contained in this letter and, to the maximum extent permitted by law, the Authority disclaims all liability and responsibility for any direct or indirect loss or damage which may be suffered by any recipient or other person through relying on anything contained in or omitted from this letter.
7. This letter has been prepared for the sole use by the party to whom it is addressed and no responsibility is accepted by the Authority with regard to any third party use for the whole or any part of its contents. Neither the whole nor any part of this letter or any reference thereto may be included in any document, circular or statement without the Authority's written approval of the form and context in which it will appear.
8. The flood information provided represents the best estimates based on currently available information. This information is subject to change as new information becomes available and as further studies are carried out.

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Connecting rivers, landscapes, people

Julia Baumann

From: Nathan Treloar <nathan.treloar@nccma.vic.gov.au>
Sent: Tuesday, March 25, 2025 9:48 AM
To: Julia Baumann
Cc: Alex Simmons; Symon Grasby
Subject: RE: [#24101-Tragowel BESS] Tragowel BESS Proposed Layout Plan

Follow Up Flag: Follow up
Flag Status: Completed

Hi Julia

Please find groundwater information below, received from my colleague Phil Dyson.

I have had a look at the range of bores present in that area. There are several, but in particular bores 125741, 20298, 6433, and 40292. All four are within the immediate area you describe.

The bad news is depth to groundwater ranges from 1 to 2 metres from the surface almost all of the time. At present given the wet conditions established by the October 22 floods, heavy rainfall in late 2023/2024, and La Nina conditions generally over the past three years we could expect depths to reflect the current shallow watertables now found across most of Tragowel Plains a little further to the south.

Groundwater salinity is likely to be extremely high, of the order of 30,000 uS/cm or greater.

I hope this information is useful, it is taken largely from the Visualising Victorias Groundwater database, and from experience gained through the current CMA bore monitoring network.

Regards Phil

Phil Dyson
Project Manager
Phil.Dyson@nccma.vic.gov.au
Ph: 5440 1886 M: +61 408 580 830



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Regards,

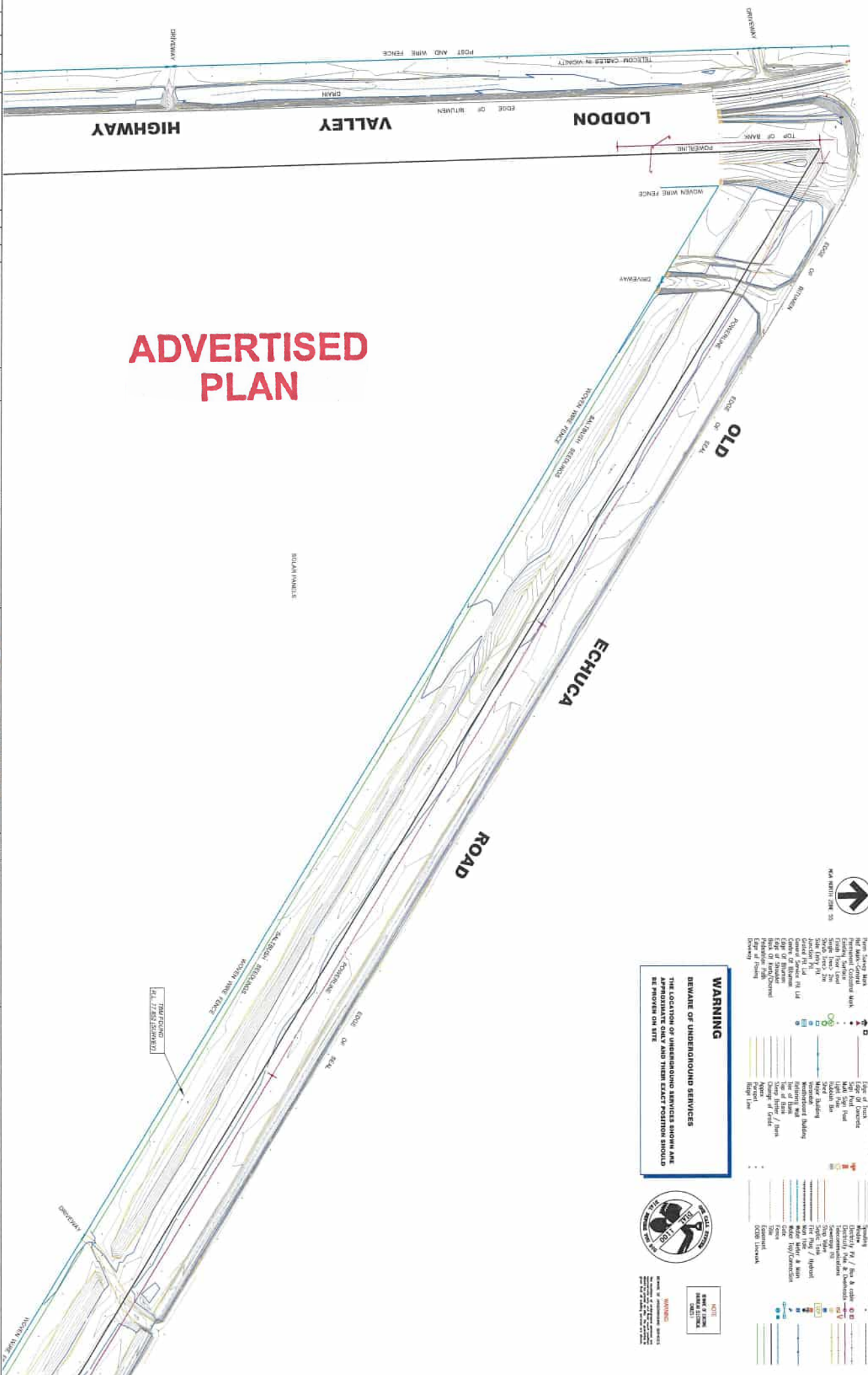
Nathan Treloar
Project Manager Waterways and Floodplain
Ph: 5440 1878 M: +61 417 131 093



From: Julia Baumann <JuliaB@dceng.com.au>
Sent: Wednesday, 26 February 2025 4:39 PM
To: Nathan Treloar <nathan.treloar@nccma.vic.gov.au>

Appendix B Feature and Level Survey

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[illegible]

WARNING

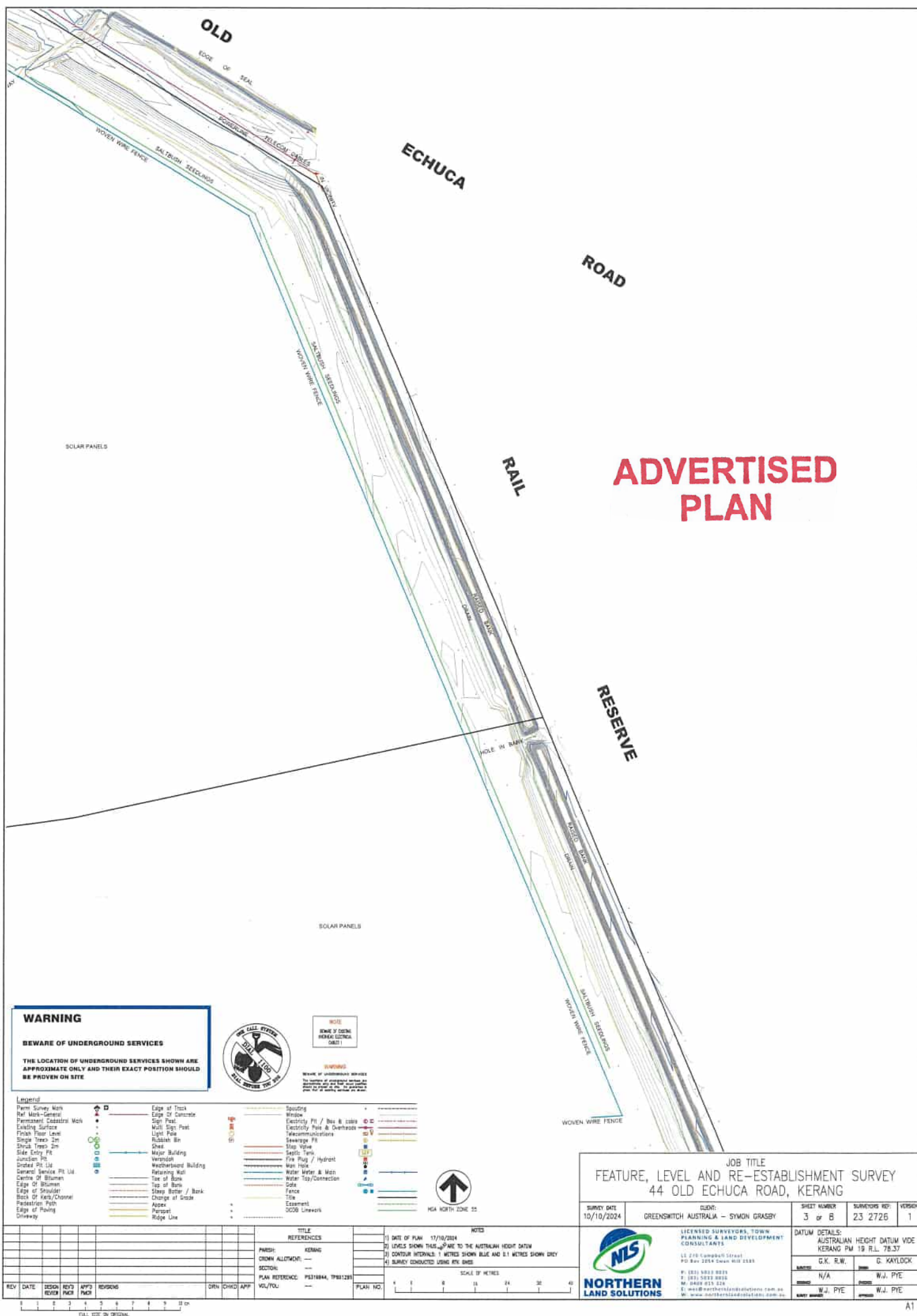
Beware of Underground Services

The location of underground services shown are approximate only and their exact position should be proven on site



NOTE
 (NAME OF EXAMINEE)
 (NAME OF DISTRICT)
 (CITY)

[illegible]



LODDON VALLEY HIGHWAY

SOLAR PANELS

SOLAR PANELS

BEWARE OF UNDERGROUND SERVICES

THE LOCATION OF UNDERGROUND SERVICES SHOWN ARE APPROXIMATE ONLY AND THEIR EXACT POSITION SHOULD BE PROVEN ON SITE



NOTE:
IMAGE OF THE
ON-BOARD ELECTRIC
CIRCUIT.

WARNING
BEWARE OF UNDERGROUND SERVICES
The locations of underground services are approximate only and field work verification should be given or else. In particular to note that all service entries are shown.

Perm Survey Mark	Edge of Terrace	Squaring
Ref Mark-General	Edge of Concrete	Know
Permanent Cathedral Mark	Set	Electricity P / Box & cable
Existing Surface	Multi Sign Post	Electricity Pole & Overhead
Finish Floor Level	Light Pole	Telecommunications
Single Tree	Roofline Set	Telephone Pit
In Struck Driveway	Shed	Stop Valve
Side Entry Pit	Major Building	Septic Tank
Junction Pit	Verandah	Fire Plug / Hydrant
Grated Pit Lin	Weatherboard Building	Van Hole
Damaged Service Pit Lin	Retaining Wall	Water Meter & Mains
Centre Of Blummen	Top of Bank	Water Tap/Connection
Edge Of Blummen	Stone Barrier / Bank	Gate
Edge of Shoulder	Change of Grade	Fence
Block Of Area/Channel	Aspen	Fence
Pedestrian Path	Perspect	Essment
Edge of Paving	Downs	OCOB Linework

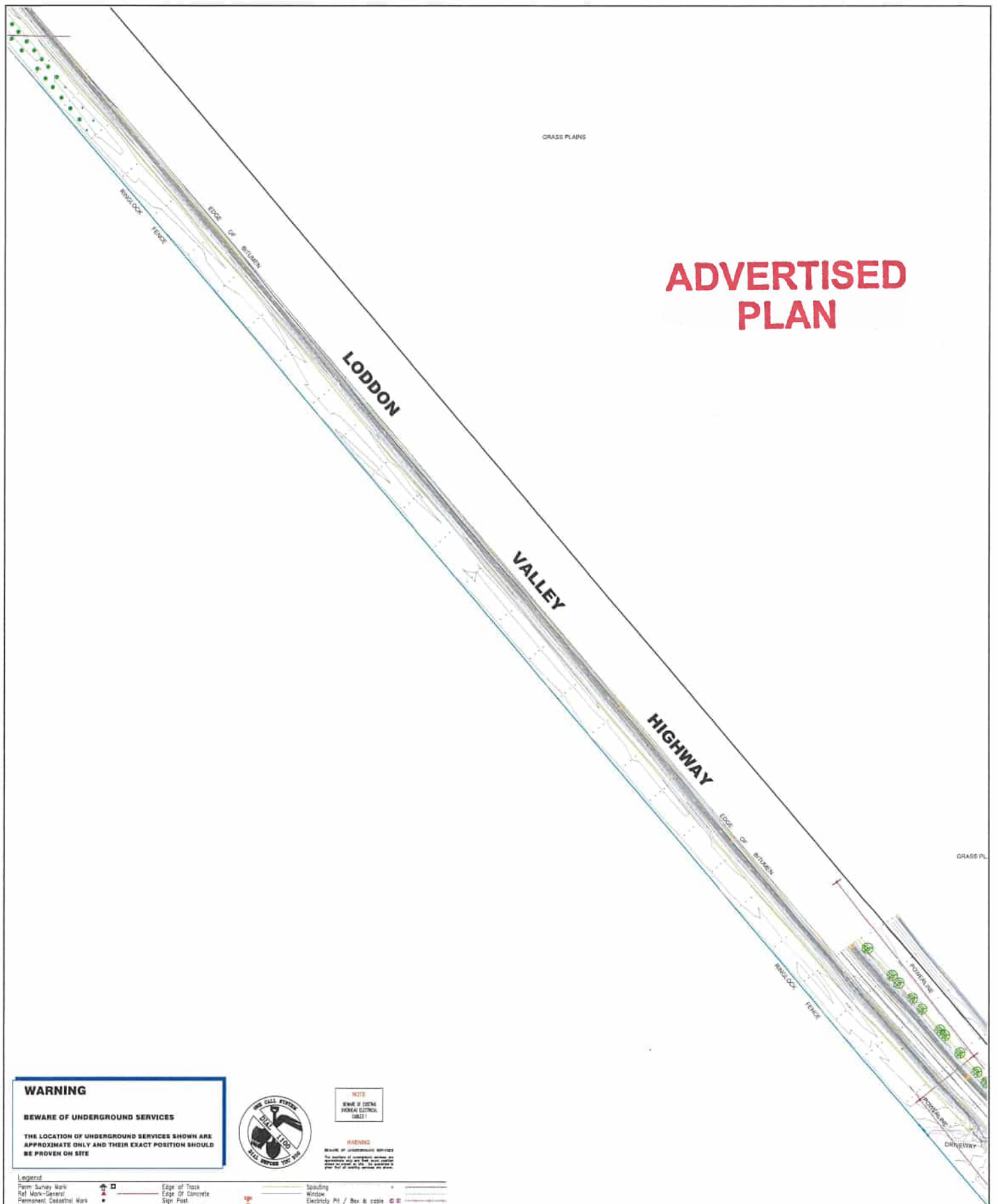


HGA NORTH ZONE 5A

JOB TITLE
FEATURE, LEVEL AND RE-ESTABLISHMENT SURVEY
44 OLD ECHUCA ROAD, KERANG

SURVEY DATE 10/10/2024	CLIENT GREENSWITCH AUSTRALIA – SYMON GRASSY	SHEET NUMBER 5 of 8	SURVEYOR REF. 23 2726	VERSION 1
 LICENSED SURVEYORS, TOWN PLANNING & LAND DEVELOPMENT CONSULTANTS 11 270 Campbell Street PO Box 1854 Swain Hill 5355 PH: (08) 5032 0019 F: (08) 5032 0026 0400 615 526 E: info@nlsolutions.com.au NORTHERN LAND SOLUTIONS		DATUM DETAILS: AUSTRALIAN HEIGHT DATUM VIDE KERANG PM 19.17 78.37		
		G.K. R.W.	G. KAYLOCK	
		N/A	N.W. PJE	
		N.W. PJE	N.W. PJE	

ADVERTISED PLAN



WARNING

BEWARE OF UNDERGROUND SERVICES

THE LOCATION OF UNDERGROUND SERVICES SHOWN ARE APPROXIMATE ONLY AND THEIR EXACT POSITION SHOULD BE PROVEN ON SITE



NOTE
Beware of underground services (pipes, cables, etc.)

WARNING
Beware of underground services (pipes, cables, etc.)

Legend	Symbol	Description
Form Survey Mark	+	Edge of Track
Ref Mark-General	+	Edge of Concrete
Permanent Cadastral Mark	+	Sign Post
Existing Surface	+	Wall Stop Post
Finish Floor Level	+	Light Pole
Single Tree- 2m	+	Rubbish Bin
Shrub Tree- 2m	+	Shed
Side Entry Pit	+	Major Building
Junction Pit	+	Verandah
Grated Pit Lid	+	Weatherboard Building
General Service Pit Lid	+	Retaining Wall
Centre Of Blummen	+	Top of Bank
Edge Of Blummen	+	Slope Butler / Bank
Back Of Yard/Channel	+	Change of Grade
Drainage Path	+	Apex
Edge of Paving	+	Parapet
Driveway	+	Ridge Line
	+	Spouting
	+	Window
	+	Electricity Pit / Box & cable
	+	Electricity Pole & Overhead
	+	Telecommunications
	+	Sewerage Pit
	+	Stop Valve
	+	Septic Tank
	+	Fire Plug / Hydrant
	+	Man Hole
	+	Water Meter & Main
	+	Water Tap/Connection
	+	Gate
	+	Fence
	+	Title
	+	Easement
	+	DCSB Linework

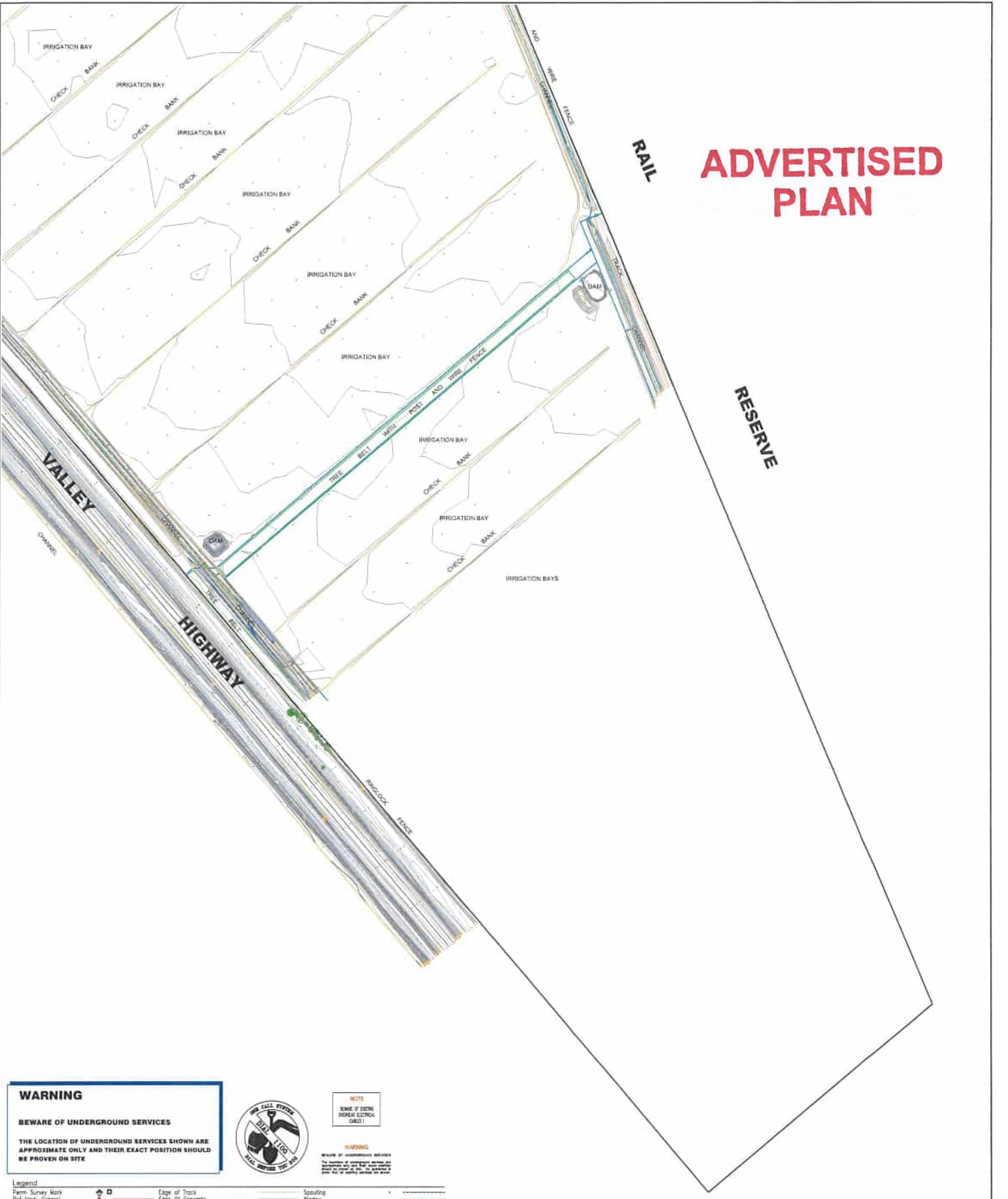


NGA NORTH 2011 55

JOB TITLE
FEATURE, LEVEL AND RE-ESTABLISHMENT SURVEY
44 OLD ECHUCA ROAD, KERANG

SURVEY DATE	CLIENT	SHEET NUMBER	SURVEYING REF.	VERSION
10/10/2024	GREENSWITCH AUSTRALIA - SYMON GRASEY	6 of 8	23.2726	1
LICENSED SURVEYORS, TOWN PLANNING & LAND DEVELOPMENT CONSULTANTS 13 210 Campbell Street PO Box 1054 Swan Hill VIC 3582 P: (03) 5033 0020 F: (03) 5033 0020 M: 0428 555 228 E: nls@nlslanddevelopment.com.au W: www.nlslanddevelopment.com.au				
DATUM DETAILS: AUSTRALIAN HEIGHT DATUM VIDE KERANG FM 19 R.L. 78.37 BY: G.K. R.W. DATE: G. KAYLOCK BY: N/A DATE: W.J. PYE BY: W.J. PYE DATE: W.J. PYE				

REV	DATE	BY	CHKD	APP	DESCRIPTION
1	10/10/2024	W.J. PYE	W.J. PYE	W.J. PYE	ISSUED FOR ADVERTISING
2	10/10/2024	W.J. PYE	W.J. PYE	W.J. PYE	REVISED TO INCLUDE COMMENTS
3	10/10/2024	W.J. PYE	W.J. PYE	W.J. PYE	REVISED TO INCLUDE COMMENTS
4	10/10/2024	W.J. PYE	W.J. PYE	W.J. PYE	REVISED TO INCLUDE COMMENTS
5	10/10/2024	W.J. PYE	W.J. PYE	W.J. PYE	REVISED TO INCLUDE COMMENTS
6	10/10/2024	W.J. PYE	W.J. PYE	W.J. PYE	REVISED TO INCLUDE COMMENTS
7	10/10/2024	W.J. PYE	W.J. PYE	W.J. PYE	REVISED TO INCLUDE COMMENTS
8	10/10/2024	W.J. PYE	W.J. PYE	W.J. PYE	REVISED TO INCLUDE COMMENTS
9	10/10/2024	W.J. PYE	W.J. PYE	W.J. PYE	REVISED TO INCLUDE COMMENTS
10	10/10/2024	W.J. PYE	W.J. PYE	W.J. PYE	REVISED TO INCLUDE COMMENTS



WARNING

BEWARE OF UNDERGROUND SERVICES

THE LOCATION OF UNDERGROUND SERVICES SHOWN ARE APPROXIMATE ONLY AND THEIR EXACT POSITION SHOULD BE PROVEN ON SITE



NOTE

NAME OF EIGHT BOREHOLE DRILL

WARNING

BEWARE OF UNDERGROUND SERVICES

Legend									
Perm. Survey Mark	Edge of Track	Spalling	Window						
Ref. Mark-General	Edge of Concrete	Electricity Pit / Box & cable	Electricity Pole & Overhead						
Permanent Cadastral Mark	Sign Post	Telecommunications	Sewerage Pit						
Existing Surface	Multi Stop Post	Stop Valve	Septic Tank						
Finish Floor Level	Light Pole	Fire Plug / Hydrant	Man Hole						
Single Track 2m	Rubbish Bin	Water Meter & Main	Water Tap/Connection						
Shrub/Tree 2m	Shed	Gate	Fence						
Side Entry Pit	Major Building	Tile	Easement						
Junction Pit	Weatherboard Building	DCGS Linework							
Grated Pit Lid	Verandah								
General Service Pit Lid	Ratcatching Wall								
Centre Of Blummen	Top of Bank								
Edge Of Blummen	Steep Batter / Bank								
Back Of Kerb/Channel	Change of Grade								
Pedestrian Path	Apex								
Edge of Paving	Pyramid								
Drainage	Ridge Line								



JOB TITLE

FEATURE, LEVEL AND RE-ESTABLISHMENT SURVEY
44 OLD ECHUCA ROAD, KERANG

SURVEY DATE 10/10/2024	CLIENT GREENSWITCH AUSTRALIA - SYMON GRASBY	SHEET NUMBER 8 of 8	SURVEYORS REF 23 2726	VERSION 1
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NORTHERN LAND SOLUTIONS

Licensed Surveyors, Town Planning & Land Development Consultants

11 218 Campbell Street
PO Box 1054 Swan Hill VIC 3585

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F: (03) 5022 9024
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W: www.northernlandsolutions.com.au

DATUM DETAILS: AUSTRALIAN HEIGHT DATUM VIDE KERANG PM 19 R.L. 78.37	DRAWN G.K. R.W.	CHECKED G. KAYLOCK
	DATE N/A	DATE W.J. PYE
	DATE W.J. PYE	DATE W.J. PYE

TITLE REFERENCES									
KERANG									
CROWN ALLOWANCE: ---									
SECTION: ---									
PLAN REFERENCE: PS18844, TP81285									
VOL/REL: ---									
PLAN NO. ---									
REV	DATE	REASON	REV'D	APPROVED	DRN	CHKD	APP		
1	10/10/2024	ISSUED FOR TENDERS	1						

1) DATE OF PLAN: 17/10/2024

2) LEVELS SHOWN THIS PLAN ARE TO THE AUSTRALIAN HEIGHT DATUM

3) CONTOUR INTERVALS: 1 METRES SHOWN BLUE AND 0.1 METRES SHOWN GREY

4) SURVEY CONDUCTED USING RTK GNSS

SCALE OF METRES

0 1 2 3 4 5 6 7 8 9 10

FULL SIZE SH ORIGINAL

Appendix C Drainage Calculations

**ADVERTISED
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**DALTON
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Stormwater Calculations

Tragowel BESS

Revision B - December 2025

**ADVERTISED
PLAN**



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ABN 78 429 221 049

CLIMATE CHANGE FACTORS										
Rainfall Factors	Choose SSP Scenario:	SSP5-8.5								
	Time Horizon	2060								
Year	<1 hour	1.5 Hours	2 Hours	3 Hours	4.5 Hours	6 Hours	9 Hours	12 Hours	18 Hours	>24 Hours
2030	1.20	1.18	1.17	1.16	1.14	1.13	1.13	1.12	1.11	1.11
2040	1.26	1.24	1.22	1.20	1.18	1.17	1.16	1.15	1.14	1.14
2050	1.34	1.31	1.29	1.26	1.24	1.23	1.21	1.20	1.18	1.18
2060	1.42	1.38	1.35	1.32	1.29	1.28	1.26	1.24	1.22	1.21
2070	1.52	1.47	1.43	1.40	1.36	1.34	1.31	1.29	1.27	1.26
2080	1.63	1.57	1.52	1.48	1.43	1.40	1.37	1.35	1.33	1.31
2090	1.77	1.69	1.64	1.58	1.52	1.49	1.45	1.42	1.39	1.37
2100	1.86	1.77	1.71	1.64	1.58	1.54	1.50	1.47	1.43	1.41

PASTE TEMPERATURE CHANGE DATA HERE				
Temperature Changes (Relative to 1961-1990 Baseline)				
Year	SSP1-2.6	SSP2-4.5	SSP3-7.0	SSP5-8.5
2030	1.20	1.20	1.20	1.30
2040	1.30	1.40	1.50	1.60
2050	1.40	1.70	1.80	2.10
2060	1.50	1.90	2.20	2.50
2070	1.50	2.10	2.50	3.00
2080	1.50	2.20	2.90	3.50
2090	1.50	2.40	3.30	4.10
2100	1.40	2.50	3.60	4.50

PASTE CLIMATE CHANGE FACTOR DATA HERE										
CLIMATE CHANGE FACTORS SSP1 - 2.6										
Year	<1 hour	1.5 Hours	2 Hours	3 Hours	4.5 Hours	6 Hours	9 Hours	12 Hours	18 Hours	>24 Hours
2030	1.18	1.17	1.16	1.14	1.13	1.12	1.12	1.11	1.10	1.10
2040	1.21	1.19	1.17	1.16	1.15	1.14	1.13	1.12	1.11	1.11
2050	1.22	1.20	1.18	1.17	1.15	1.15	1.14	1.13	1.12	1.11
2060	1.23	1.21	1.20	1.18	1.17	1.16	1.15	1.14	1.13	1.12
2070	1.24	1.22	1.20	1.18	1.17	1.16	1.15	1.14	1.13	1.12
2080	1.23	1.21	1.20	1.18	1.17	1.16	1.15	1.14	1.13	1.12
2090	1.23	1.21	1.20	1.18	1.17	1.16	1.15	1.14	1.13	1.12
2100	1.22	1.20	1.19	1.17	1.16	1.15	1.14	1.13	1.12	1.12

CLIMATE CHANGE FACTORS SSP2 - 4.5										
Year	<1 hour	1.5 Hours	2 Hours	3 Hours	4.5 Hours	6 Hours	9 Hours	12 Hours	18 Hours	>24 Hours
2030	1.18	1.17	1.16	1.14	1.13	1.12	1.12	1.11	1.10	1.10
2040	1.22	1.20	1.19	1.17	1.16	1.15	1.14	1.13	1.12	1.12
2050	1.27	1.24	1.23	1.21	1.19	1.18	1.17	1.16	1.15	1.14
2060	1.30	1.27	1.25	1.23	1.21	1.20	1.19	1.18	1.16	1.16
2070	1.33	1.30	1.28	1.26	1.24	1.22	1.21	1.19	1.18	1.17
2080	1.37	1.33	1.31	1.28	1.26	1.24	1.22	1.21	1.20	1.19
2090	1.40	1.36	1.34	1.31	1.28	1.26	1.24	1.23	1.21	1.20
2100	1.41	1.37	1.35	1.32	1.29	1.27	1.25	1.24	1.22	1.21

CLIMATE CHANGE FACTORS SSP3 - 7.0										
Year	<1 hour	1.5 Hours	2 Hours	3 Hours	4.5 Hours	6 Hours	9 Hours	12 Hours	18 Hours	>24 Hours
2030	1.18	1.17	1.16	1.14	1.13	1.12	1.12	1.11	1.10	1.10
2040	1.23	1.21	1.20	1.18	1.17	1.16	1.15	1.14	1.13	1.12
2050	1.29	1.26	1.24	1.22	1.20	1.19	1.18	1.17	1.16	1.15
2060	1.35	1.32	1.30	1.27	1.25	1.23	1.22	1.20	1.19	1.18
2070	1.42	1.38	1.35	1.32	1.29	1.28	1.26	1.24	1.22	1.21
2080	1.50	1.45	1.42	1.38	1.35	1.33	1.30	1.28	1.26	1.25
2090	1.59	1.53	1.49	1.44	1.40	1.38	1.35	1.33	1.30	1.29
2100	1.66	1.59	1.55	1.50	1.45	1.42	1.39	1.37	1.34	1.32

CLIMATE CHANGE FACTORS SSP5 - 8.5										
Year	<1 hour	1.5 Hours	2 Hours	3 Hours	4.5 Hours	6 Hours	9 Hours	12 Hours	18 Hours	>24 Hours
2030	1.20	1.18	1.17	1.16	1.14	1.13	1.13	1.12	1.11	1.11
2040	1.26	1.24	1.22	1.20	1.18	1.17	1.16	1.15	1.14	1.14
2050	1.34	1.31	1.29	1.26	1.24	1.23	1.21	1.20	1.18	1.18
2060	1.42	1.38	1.35	1.32	1.29	1.28	1.26	1.24	1.22	1.21
2070	1.52	1.47	1.43	1.40	1.36	1.34	1.31	1.29	1.27	1.26
2080	1.63	1.57	1.52	1.48	1.43	1.40	1.37	1.35	1.33	1.31
2090	1.77	1.69	1.64	1.58	1.52	1.49	1.45	1.42	1.39	1.37
2100	1.86	1.77	1.71	1.64	1.58	1.54	1.50	1.47	1.43	1.41

Time (min)	2030	2040	2050	2060	2070	2080	2090	2100
0	1.2	1.26	1.34	1.42	1.52	1.63	1.77	1.86
60	1.2	1.26	1.34	1.42	1.52	1.63	1.77	1.86
90	1.18	1.24	1.31	1.38	1.47	1.57	1.69	1.77
120	1.17	1.22	1.29	1.35	1.43	1.52	1.64	1.71
180	1.16	1.2	1.26	1.32	1.4	1.48	1.58	1.64
270	1.14	1.18	1.24	1.29	1.36	1.43	1.52	1.58
360	1.13	1.17	1.23	1.28	1.34	1.4	1.49	1.54
540	1.13	1.16	1.21	1.26	1.31	1.37	1.45	1.5
720	1.12	1.15	1.2	1.24	1.29	1.35	1.42	1.47
1080	1.11	1.14	1.18	1.22	1.27	1.33	1.39	1.43
1440	1.11	1.14	1.18	1.21	1.26	1.31	1.37	1.41

The baseline IFDs should be adjusted to account for climate change as follows:

Parameter	Current	Future (Years 2070 & 2100)
IFD	Scale using Datahub SSP3-7.0 factors for Year 2030	Scale using ARR v4.2 Datahub SSP-based factors for SSP2-4.5, SSP3-7.0, and SSP5-8.5

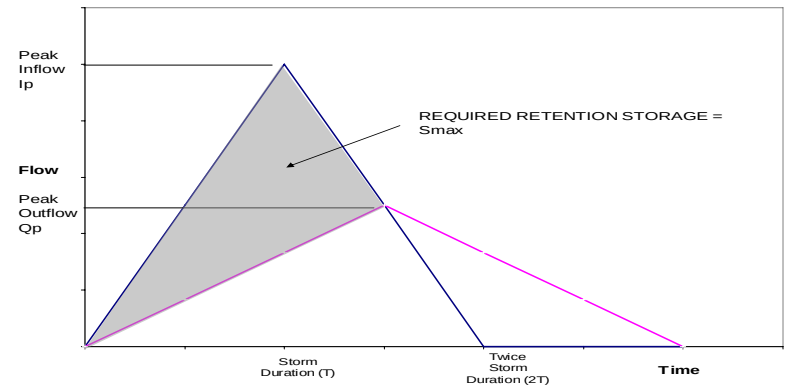
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[illegible]

Project reference 24101
Project Name Tragowel BESS
Date 18-12-25
Resource J Baumann
Description 1% Detention Storage



Catchment Area 7 ha
C (1%) 0.63
Effective area 4.41 ha
Max outflow 0.4 m³/s



STORAGE

Based on Boyds Formula:

$$S_{max} = v1(1 - \frac{Qp}{Ip})$$

Where....

Smax = Max Volume of Temporary Storage (cu,m)

V1 = Co-Efficient of Runoff

I = Volume of Inflow Flood (cu.m)

Ip = Peak Discharge of Inflow Hydrograph (cu.m/s)

Qp = Peak Discharge of Outflow Hydrograph (cu.m/s)

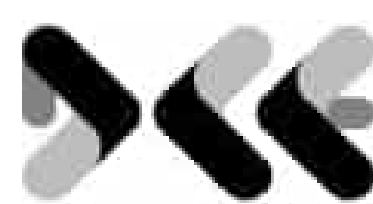
1,130

 m³ of storage required

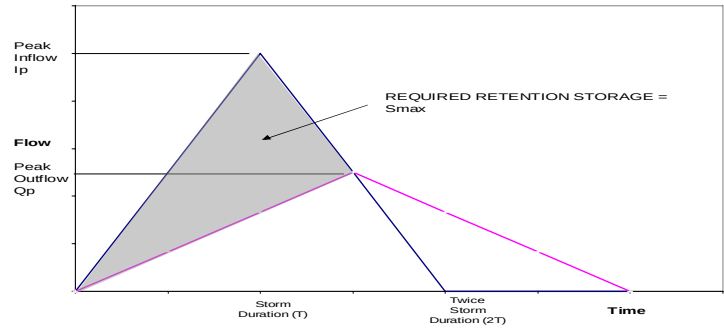
Duration (min)	1% intensity (mm/hr)	Ip	Qp	V1	Smax
1	317.092	3.884375095	0.4	233.0625	210
2	265.597	3.25356519	0.4	390.4278	340
3	239.961	2.93952513	0.4	529.1145	460
4	221.114	2.708643765	0.4	650.0745	550
5	205.833	2.521451002	0.4	756.4353	640
6	192.946	2.363591949	0.4	850.8931	710
7	181.839	2.227529522	0.4	935.5624	770
8	172.123	2.108509516	0.4	1012.085	820
9	163.529	2.003229462	0.4	1081.744	870
10	155.858	1.909263297	0.4	1145.558	910
11	148.960	1.824763103	0.4	1204.344	940
12	142.717	1.748284679	0.4	1258.765	970
13	137.035	1.678676983	0.4	1309.368	1000
14	131.837	1.615008034	0.4	1356.607	1020
15	127.062	1.556513237	0.4	1400.862	1040
16	122.658	1.502558211	0.4	1442.456	1060
17	118.581	1.452611421	0.4	1481.664	1070
18	114.794	1.406223616	0.4	1518.722	1090
19	111.266	1.363012128	0.4	1553.834	1100
19.5	109.591	1.342493075	0.4	1570.717	1100
20	107.971	1.322648706	0.4	1587.178	1110
22	101.989	1.249369891	0.4	1649.168	1120
23	99.265	1.215993636	0.4	1678.071	1130
24	96.697	1.184532765	0.4	1705.727	1130
25	94.271	1.154821195	0.4	1732.232	1130
26	91.976	1.126711928	0.4	1757.671	1130
27	89.802	1.100074342	0.4	1782.12	1130
28	87.738	1.074791939	0.4	1805.65	1130
29	85.776	1.050760457	0.4	1828.323	1130
30	83.909	1.027886275	0.4	1850.195	1130
31	82.129	1.006085078	0.4	1871.318	1130
32	80.431	0.985280704	0.4	1891.739	1120
33	78.809	0.965404174	0.4	1911.5	1120
34	77.257	0.946392848	0.4	1930.641	1110
35	75.771	0.928189708	0.4	1949.198	1110
36	74.346	0.91074273	0.4	1967.204	1100
37	72.980	0.894004341	0.4	1984.69	1100
38	71.668	0.877930949	0.4	2001.683	1090
39	70.407	0.862482529	0.4	2018.209	1080
40	69.194	0.84762226	0.4	2034.293	1070
41	68.026	0.833316209	0.4	2049.958	1070
42	66.901	0.819533046	0.4	2065.223	1060
43	65.816	0.806243801	0.4	2080.109	1050
44	64.769	0.793421639	0.4	2094.633	1040
45	63.759	0.781041667	0.4	2108.812	1030
46	62.782	0.769080757	0.4	2122.663	1020
47	61.838	0.757517396	0.4	2136.199	1010
48	60.925	0.746331537	0.4	2149.435	1000
49	60.041	0.735504485	0.4	2162.383	990
50	59.185	0.725018775	0.4	2175.056	980
55	55.279	0.677173541	0.4	2234.673	910
60	51.902	0.635799327	0.4	2288.878	850
65	48.950	0.599638729	0.4	2338.591	780
70	46.346	0.56774383	0.4	2384.524	700
75	44.031	0.539385831	0.4	2427.236	630

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Project reference 24101
Project Name Tragowel BESS
Date 18-12-25
Resource J Baumann
Description 1% Detention Storage--Climate Change



Catchment Area 7 ha
C (1%) 0.63
Effective area 4.41 ha
Max outflow 0.6 m³/s



STORAGE

Based on Boyds Formula:

$$S_{max} = v1(1 - \frac{Qp}{Ip})$$

Where....

Smax = Max Volume of Temporary Storage (cu,m)

V1 = Co-Efficient of Runoff

I = Volume of Inflow Flood (cu.m)

Ip = Peak Discharge of Inflow Hydrograph (cu.m/s)

Qp = Peak Discharge of Outflow Hydrograph (cu.m/s)

1560

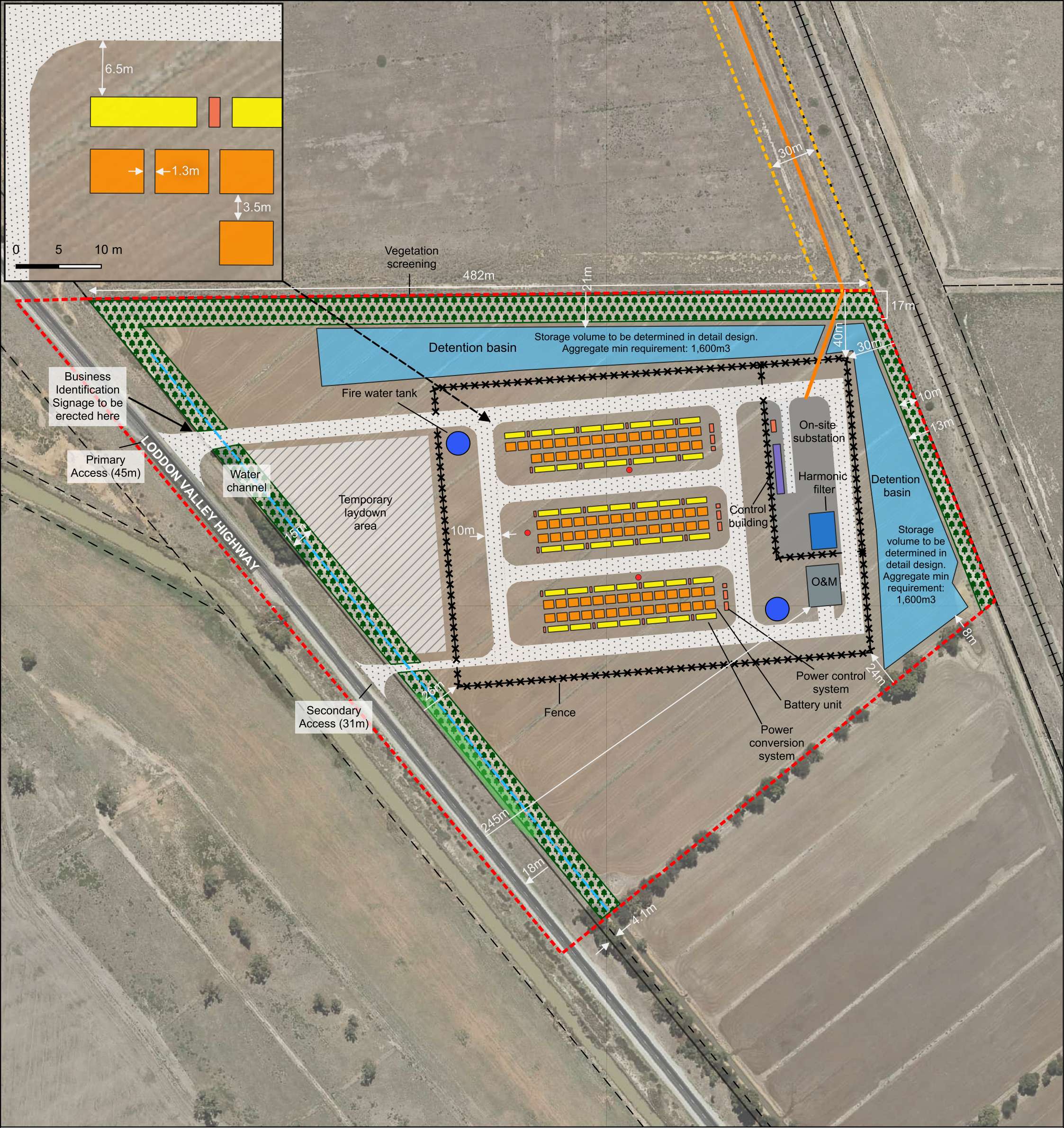
 m³ of storage required

1% intensity (mm/hr)		Ip	Qp	V1	Smax
Duration (min)	Adjusted for climate change				
1	450.270	5.515812635	0.6	330.9488	290
2	377.148	4.620062569	0.6	554.4075	480
3	340.745	4.174125685	0.6	751.3426	640
4	313.982	3.846274146	0.6	923.1058	780
5	292.282	3.580460423	0.6	1074.138	890
6	273.984	3.356300568	0.6	1208.268	990
7	258.212	3.163091921	0.6	1328.499	1080
8	244.415	2.994083512	0.6	1437.16	1150
9	232.211	2.844585837	0.6	1536.076	1210
10	221.319	2.711153882	0.6	1626.692	1270
11	211.524	2.591163606	0.6	1710.168	1310
12	202.658	2.482564244	0.6	1787.446	1360
13	194.589	2.383721315	0.6	1859.303	1390
14	187.209	2.293311409	0.6	1926.382	1420
15	180.428	2.210248796	0.6	1989.224	1450
16	174.174	2.133632659	0.6	2048.287	1470
17	168.384	2.062708217	0.6	2103.962	1490
18	163.007	1.996837535	0.6	2156.585	1510
19	157.998	1.935477222	0.6	2206.444	1520
19.5	155.620	1.906340166	0.6	2230.418	1530
20	153.319	1.878161163	0.6	2253.793	1530
22	144.825	1.774105245	0.6	2341.819	1550
23	140.956	1.726710963	0.6	2382.861	1550
24	137.309	1.682036527	0.6	2422.133	1560
25	133.865	1.639846097	0.6	2459.769	1560
26	130.607	1.599930937	0.6	2495.892	1560
27	127.519	1.562105565	0.6	2530.611	1560
28	124.588	1.526204554	0.6	2564.024	1560
29	121.802	1.492079849	0.6	2596.219	1550
30	119.151	1.459598511	0.6	2627.277	1550
31	116.624	1.428640811	0.6	2657.272	1540
32	114.212	1.3990986	0.6	2686.269	1530
33	111.908	1.370873927	0.6	2714.33	1530
34	109.704	1.343877844	0.6	2741.511	1520
35	107.594	1.318029386	0.6	2767.862	1510
36	105.572	1.293254676	0.6	2793.43	1500
37	103.632	1.269486164	0.6	2818.259	1490
38	101.768	1.246661947	0.6	2842.389	1470
39	99.978	1.224725191	0.6	2865.857	1460
40	98.255	1.203623609	0.6	2888.697	1450
41	96.597	1.183309017	0.6	2910.94	1430
42	94.999	1.163736926	0.6	2932.617	1420
43	93.458	1.144866198	0.6	2953.755	1410
44	91.972	1.126658727	0.6	2974.379	1390
45	90.537	1.109079166	0.6	2994.514	1370
46	89.151	1.092094675	0.6	3014.181	1360
47	87.810	1.075674702	0.6	3033.403	1340
48	86.514	1.059790783	0.6	3052.197	1320
49	85.258	1.044416369	0.6	3070.584	1310
50	84.043	1.029526661	0.6	3088.58	1290
55	78.497	0.961586428	0.6	3173.235	1190
60	73.701	0.902835045	0.6	3250.206	1090
65	69.509	0.851486996	0.6	3320.799	980
70	65.812	0.806196238	0.6	3386.024	870
75	62.525	0.76592788	0.6	3446.675	750

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Appendix D Development Plans

**ADVERTISED
PLAN**



BESS Concept Layout Plan

2405 - Tragowel BESS

LEGEND

Project Site

- BESS Development Area
- Transmission Development Area

Proposed Infrastructure

- Underground transmission line
- Battery unit
- Power conversion system
- Power control systems

- Harmonic filter
- Control building
- Operations and maintainance
- On site substation
- Water tanks
- Vegetation screening
- Fire Hydrants
- Fence

- Temporary laydown area
- Access roads

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Existing Features

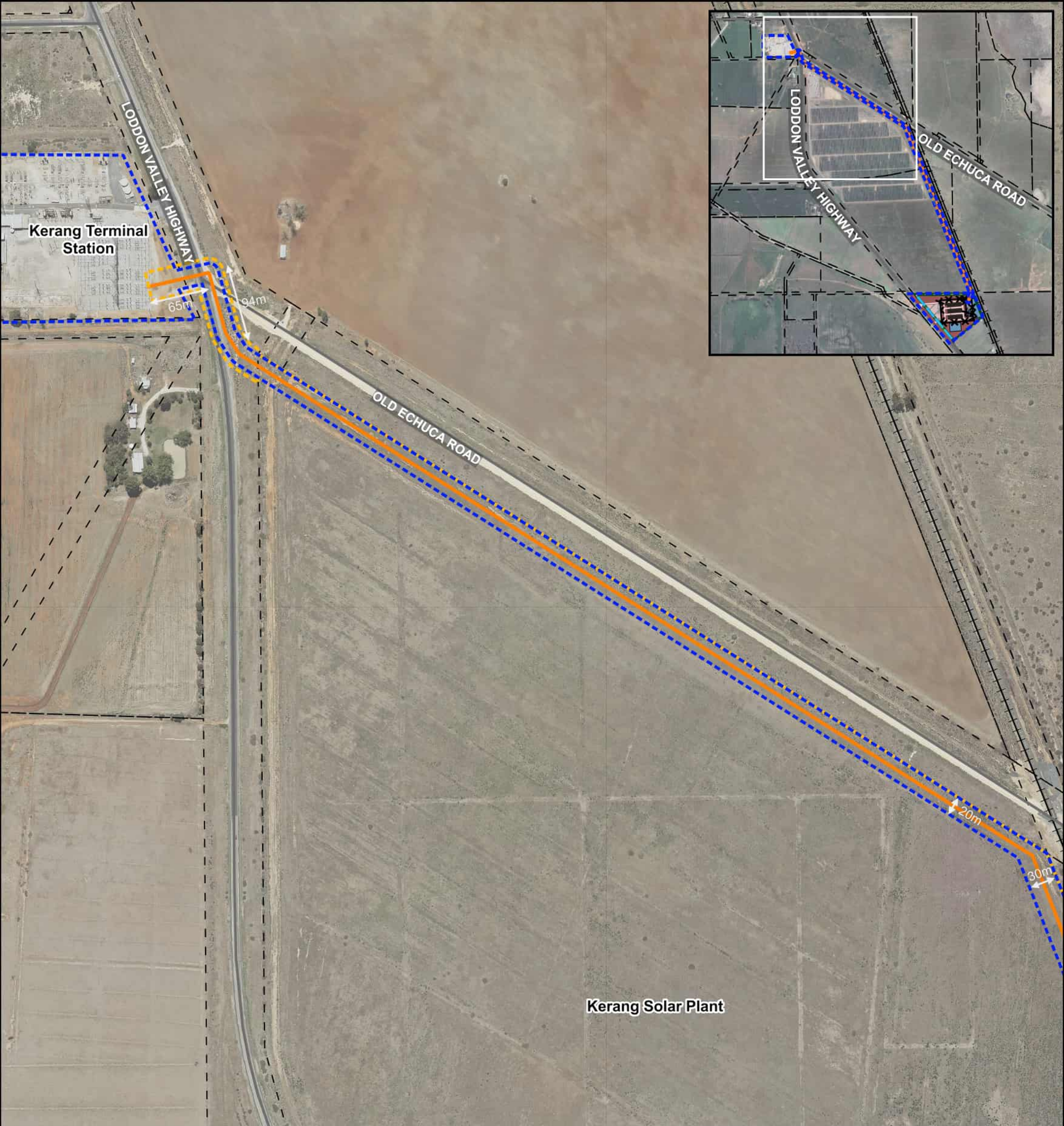
- Impacted native vegetation
- Roads
- Rail line
- Water channel
- Surveyor title boundary
- VicPlan title boundary

Version: 1.3

Date: 15/04/2026

Disclaimer: This plan is preliminary and subject to detailed studies and approval.

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Transmission route - northern extent

2405 - Tragowel BESS

LEGEND

Proposal

-  Project Site
-  Transmission Development Area
-  Transmission Cable Route

Existing Features

-  Roads
-  Rail line
-  Cadastre

0 100 200 m



Version: 1.3

Date: 14/04/2026

Disclaimer: This plan is preliminary and subject to detailed studies and approval.

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PLAN**

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Transmission route - southern extent

2405 - Tragowel BESS

0 100 200 m



LEGEND

- | | |
|--------------------------------|----------------------------|
| Project Site | Existing Features |
| Transmission Cable Route | Impacted Native Vegetation |
| BESS Development Area | Roads |
| Transmission Development Area | Rail line |
| Proposed Infrastructure | Cadastre |
| Vegetation Screening | |

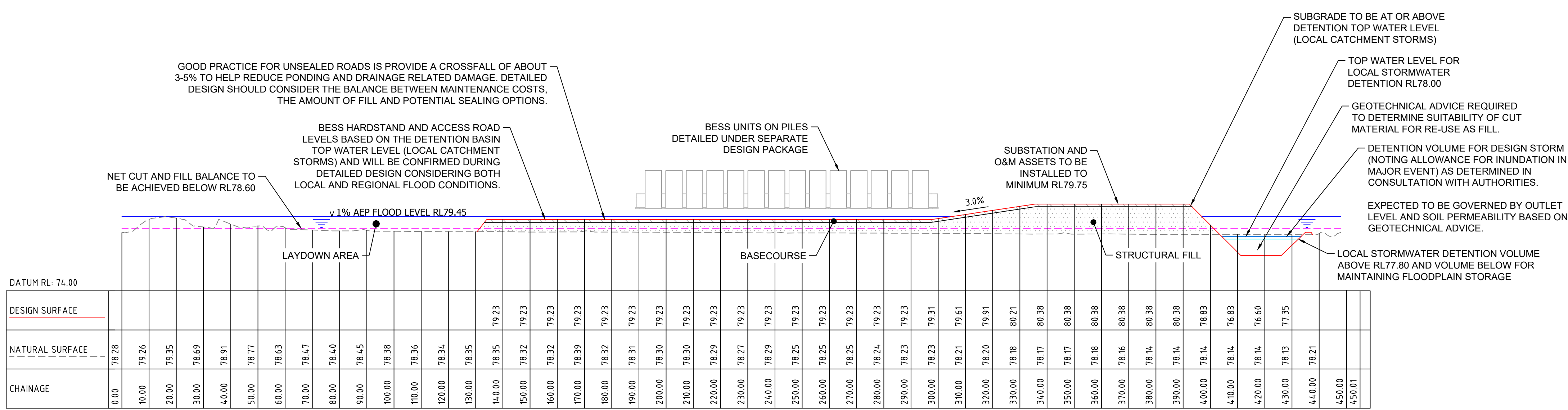
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PLAN**

Version: 1.3

Date: 14/04/2026

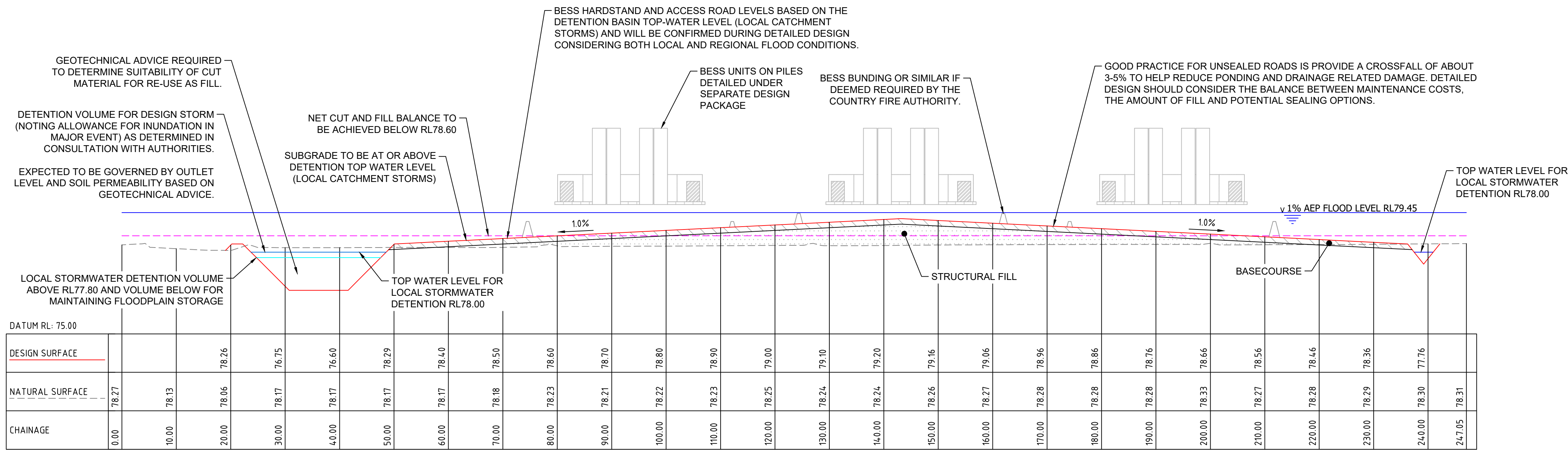
Disclaimer: This plan is preliminary and subject to detailed studies and approval.

cogency

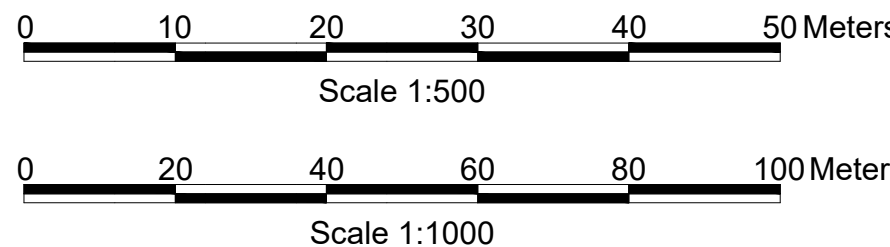
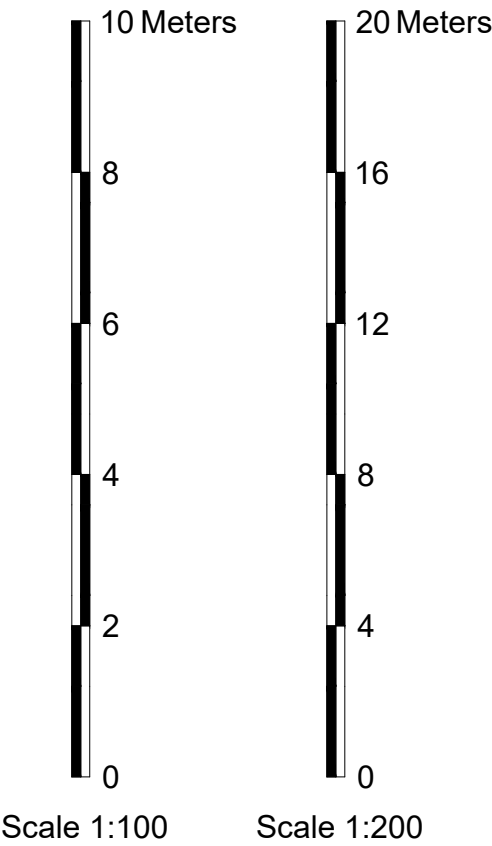


LEGEND	
(EXISTING)	
1% AEP FLOOD LEVEL	
NET CUT AND FILL LEVEL	
NATURAL SURFACE	
(PROPOSED)	
FINISHED SURFACE	
TOP WATER LEVEL	
FLOODPLAIN STORAGE LEVEL	
SUBBASE SURFACE	

SECTION A
SCALE H=1:1000, V=1:200



SECTION B
SCALE H=1:500, V=1:100



ADVERTISED
PLAN

APD GLOBAL

CLIENT		REV	DATE	DESCRIPTION	DRAWN	CHECKED	REVIEWED	VERIFIED			PROJECT		
	A	12/12/2025	PRELIMINARY ISSUE	K.NAVARRO	G.SPELDEWINDE	----					TRAGOWEL BESS PROJECT		
	B	08/01/2026	DRAWINGS UPDATED	K.NAVARRO	G.SPELDEWINDE	----			SCALE	AS SHOWN	TYPE		TYPICAL SECTIONS
	C	22/04/2026	PRELIMINARY ISSUE	K.NAVARRO	G.SPELDEWINDE	----			SIZE	A1			
									DESIGNED				
ORIGINATOR									G.SPELDEWINDE				
	CERTIFIER'S NAME				ENTITY			REG NO.	NOTE: DIMENSIONS IN METERS PRINT IN COLOR		STATUS	POTENTIA DRG NO.	TRGWLES-APDE-DRG-CVGN-0004
											DRAFT	EXTERNAL DRG NO.	30501-CI-E02