

MEL1 - Spotswood

Stormwater Management Plan

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2026-03-25

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1 Executive Summary

Aurecon was engaged by GreenSquareDC to prepare a Stormwater Management Plan (SMP) suitable for submission to the Department of Transport and Planning (DTP) for a site located at 44-76 Simcock Avenue, Spotswood. The proposal is for a data centre development, designed to support the increasing need for large scale data storage and processing in Australia's technology sector.

The purpose of this Stormwater Management Plan is to provide advice on the proposed development as detailed in the Greenbox architectural drawings. The works described herein are subject to further approvals and cover works required to service the proposed development including stormwater quality and quantity measures, flooding, and drainage.

The stormwater quantity objective was to demonstrate that there will be no increase in total peak discharge from the subject site. This considered storm events up to and including the 1%AEP storm event.

As the post-developed site will have a higher proportion of pervious area compared to the site's pre-demolition condition, there will be no increase in flow volume or rate due to development. As such, no on-site detention will be required. This expectation was also outlined within Section 4.2 of the endorsed subdivision SMP (Ref: 44-76 Simcock Av Spotswood SWS Rev8). Design philosophy outlined within the endorsed subdivision stormwater management plan is adhered to by proposed design.

In addition to the stormwater quantity results, the report includes a summary of the modelled water quality results. Water Sensitive Urban Design (WSUD) features and Council Approved Proprietary Water Quality Treatment Products have been included in the design to achieve the water quality objectives specified in the Victoria Planning Provisions 2018. This includes the removal of gross pollutants, suspended solids, nitrogen and phosphorus to target reduction levels. Aurecon recommends the use of the following devices to meet the water quality objectives identified within the VPP:

- 2x Ocean Protect CS2250 Cascade Separator's
- 1x Ocean Protect OS-2318 OceanSave Gross Pollutant Trap
- Ocean Protect OceanGuard Trash Baskets fitted within Inlet Pits

All relevant standards and guidelines are addressed in this report including criteria from:

- Develop Facilitation Program (DFP)
- Hobsons Bay Planning Scheme 2025
- Victoria Planning Provisions (VPP) 2026
- Urban Stormwater – Best Practice Environmental Management Guidelines (BPEMG) 1999
- Melbourne Water MUSIC Modelling Guidelines
- Plumbing and Drainage Code AS3500.3
- Australian Rainfall and Runoff (ARR)

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2 Introduction

2.1 Background

Aurecon was engaged by GreenSquareDC to prepare a Stormwater Management Plan suitable for submission to the Department of Transport, under the Development Facilitation Program (DFP) and any required referral agencies for a site located at 44-76 Simcock Avenue, Spotswood.

The purpose of this Stormwater Management Plan is to provide advice on the proposed development with regards to stormwater quality and quantity measures, flooding, and drainage. The required infrastructure will be subject to the conditions attached to the Planning Permit issued by Department of Transport and Planning (DTP) and any nominated referral agencies.

2.2 Property Detail

The details of the property for the proposed development can be seen in Table 1 below.

Table 1 - Property Detail

Street Address	44-76 Simcock Avenue, Spotswood
Site Area	7.996 ha

The location of the proposed development is demonstrated in Figure 1.

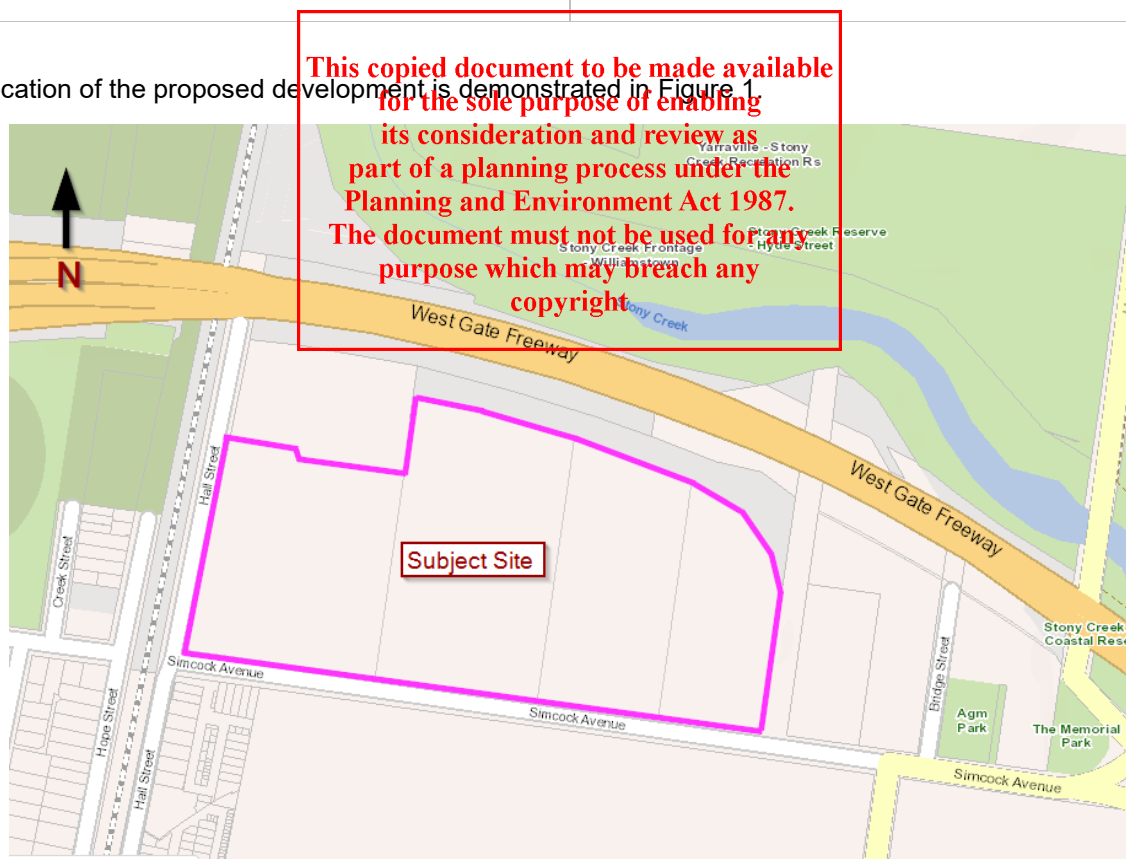


Figure 1 - Site Location (as accessed from VicPlan 09.01.26)

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3 Existing Conditions

3.1 Existing Site Features

The subject site is currently vacant with moderate grass coverage and contains a small number of small to medium sized trees.

The site is bound by:

- Westgate Freeway On-ramp to the north and east of the site;
- Simcock Avenue to the south; and
- Hall Street to the west.

The site contains a ridge within the western half of the property which separates an area that falls north-west towards Hall Street from an area falling north-east towards the Westgate Freeway On-ramp. The existing site features can be seen in Figure 2.



Figure 2 - Site Layout (as accessed from Google Earth 09.01.26)

The existing contours and surface levels can be seen on the survey plan attached within Appendix A.

Prior to demolition within the site, it was formerly used for industrial purposes with a very impervious coverage as can be seen below in Figure 3.

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Figure 3 - Site Layout Prior to Demolition (2013)

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4 Flooding and freeboard requirements

A VicPlan Property Report for the site (formally known as 44-76 Simcock Avenue, Spotswood) was generated and is attached Appendix E within for further information. As can be seen within the Property Report, a flood overlay exists along Stony Creek. However, the highest surface level within the nearest overlay extents is taken to be 5-10m lower than the lowest point within the site. Consequently, the site is not deemed to be at risk of flooding.

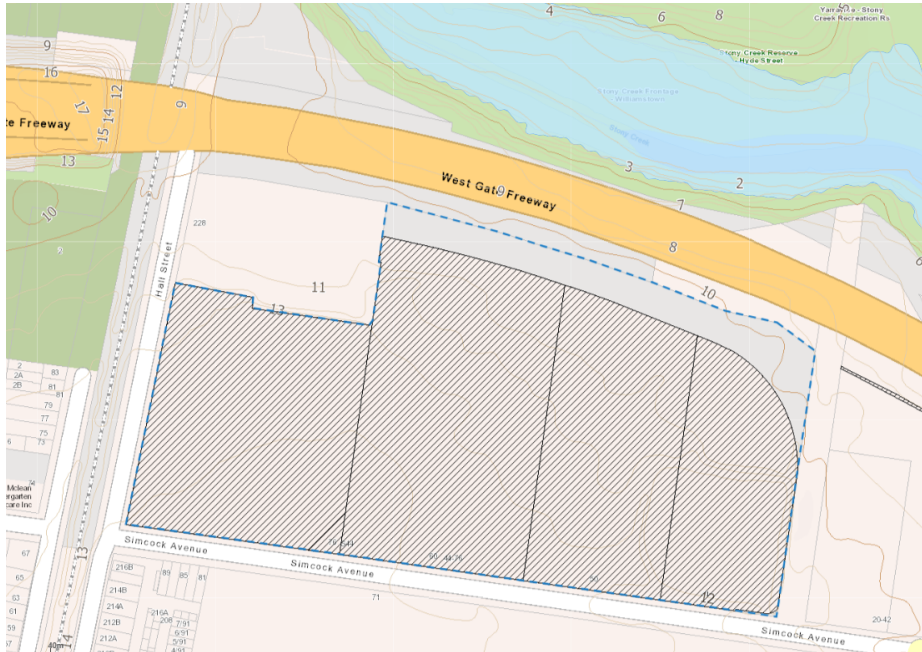


Figure 4 - Project Site with Flood Overlay

5 Stormwater Infrastructure

5.1.1 Freeboard measures

It is intended that the internal roads and site drainage system will be designed such that the Finished Floor Levels of the building and other habitable areas will be above the 1% AEP flood level and required freeboard (300mm).

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5.2 Existing Infrastructure

Aerial imagery shows the following stormwater infrastructure within the vicinity of the subject site:

- Several stormwater inlet pits and a shallow table drain falling north along Hall Street external to the site's western boundary;
- Kerb inlet pits along Hall Street external to the site's southern boundary; and
- Kerb inlet pits along the Westgate Freeway on-ramp external to the site's north-eastern boundary.

5.3 Endorsed Subdivision Stormwater Management Plan

A Stormwater Management Plan was prepared by Incitus to support an application to subdivide the site, formerly known as 44-76 Simcock Avenue, Spotswood. Hobsons Bay City Council endorsed revision 8 of the SMP on the 04/09/2025. The endorsed SMP outlined the following information regarding the proposed 4 lot subdivision, legal points of discharge (LPD's), stormwater infrastructure and catchment hydrology. A copy of the endorsed SMP can be found in Appendix D.

5.3.1 Subdivision Layout

The proposed subdivision associated with the endorsed SMP can be seen below in Figure 5.

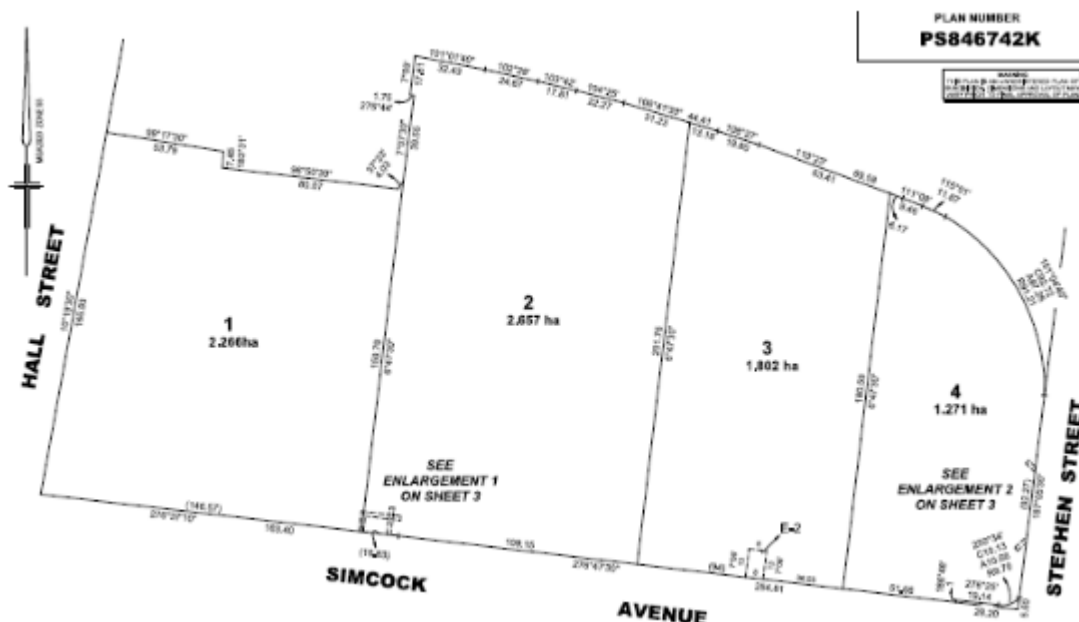


Figure 5 - Subdivision Layout

5.3.2 Proposed Legal Points of Discharge (LPD)

The proposed LPD's outlined within the endorsed SMP are summarised below:

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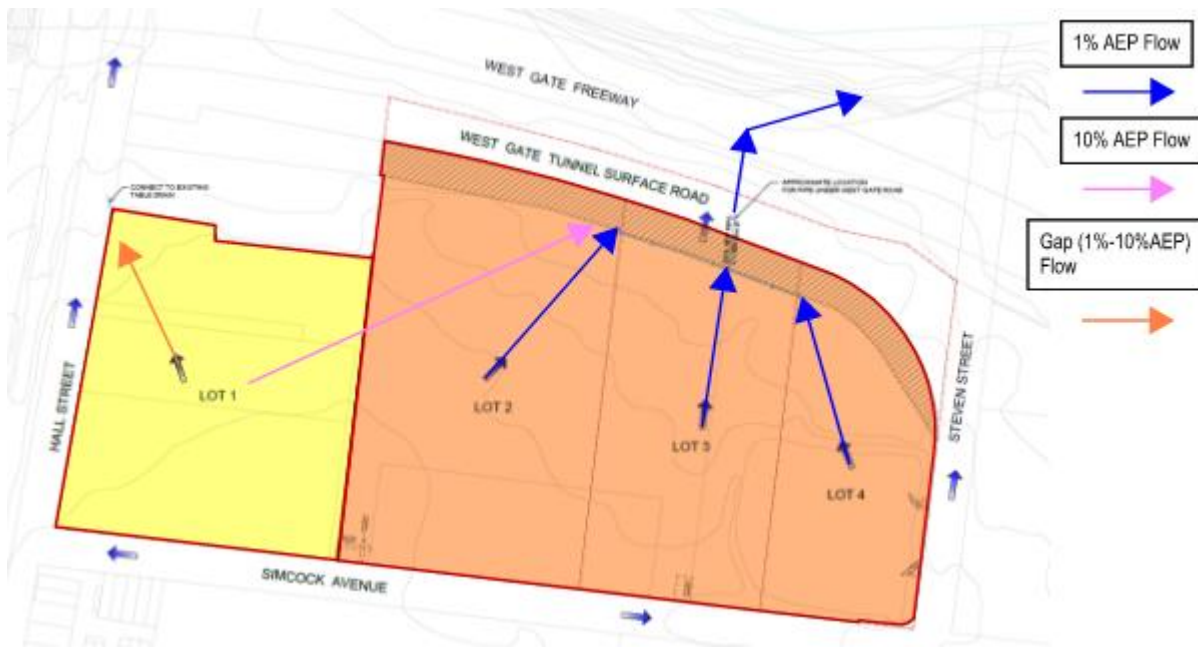


Figure 6 - Endorsed Subdivision SMP LPD's

Hall Street Table Drain

- Gap flows (flows exceeding 10%AEP pipe network capacity) from proposed Lot 1 to discharge over the western boundary into the existing table drain along the eastern side of Hall Street.

New Pipe Network Outfall to Stony Creek

- 1%AEP flow for proposed Lots 2-4 to discharge directly to Stony Creek via new outfall pipe network.
- 10%AEP flow from Lot 1 to discharge directly to Stony Creek via new outfall pipe network.

It should be noted that the endorsed strategy involves reducing the catchment area and flow discharging into the Hall Street table drain and increases the area and flow draining north-east. However, as the proposal involves the construction of a new 1%AEP pipe network discharging directly to Stony Creek, the strategy is deemed to improve the local catchment hydrology and provide a safer discharge route into Stony Creek where all flows ultimately merge.

A copy of the Approved for Construction "1% AEP Drainage Outfall Detail Plan & Longsection" can be found within the Appendix D of the endorsed SMP. A snippet of the Approved for Construction drawing can be seen below in Figure 7.

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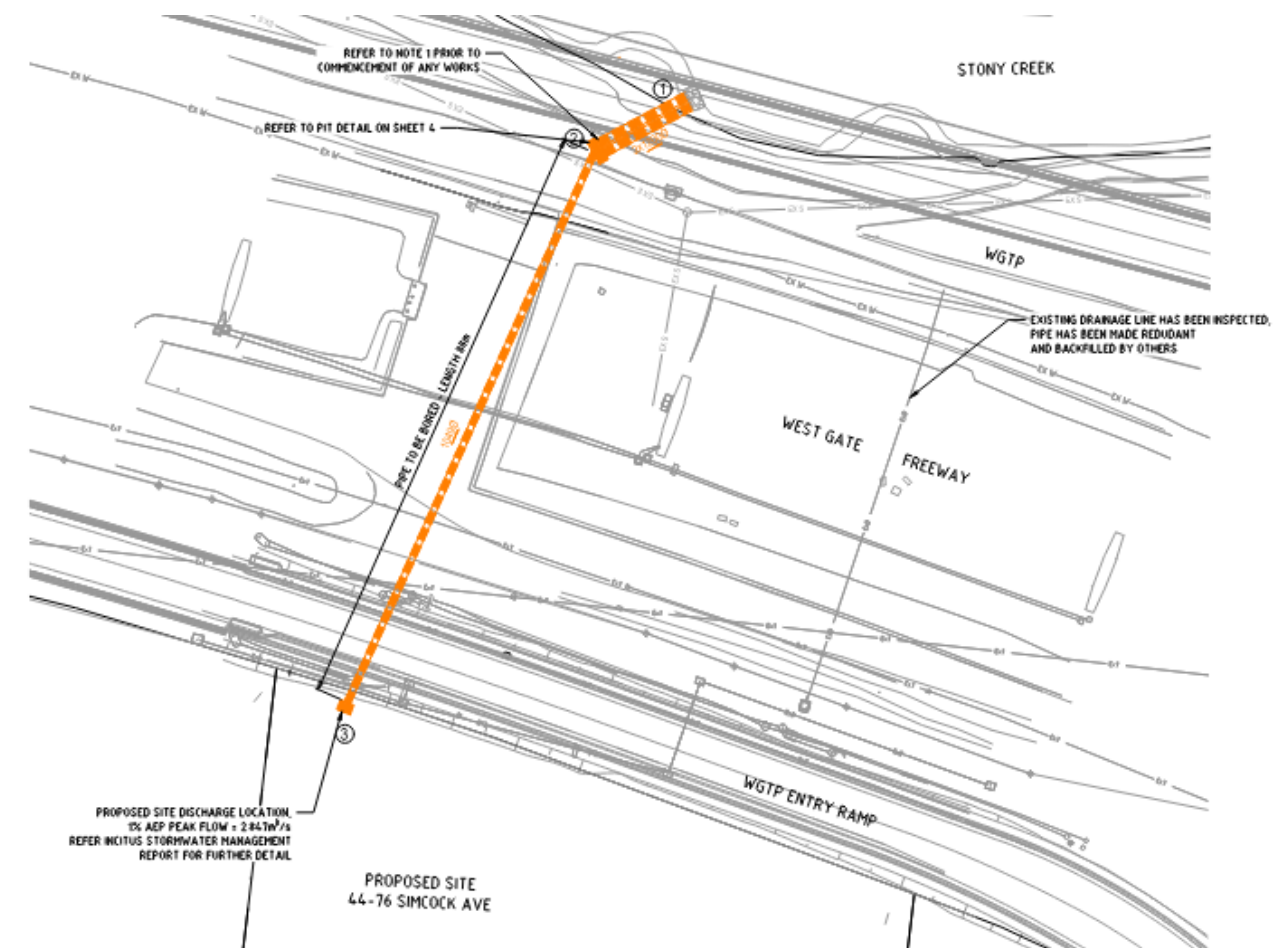


Figure 7 - Endorsed Subdivision SMP Stony Creek Outfall Arrangement

5.3.3 Proposed Catchment Hydrology

The endorsed SMP outlined the following with regard to proposed catchment hydrology:

- Estimated Developed Fraction Impervious = 0.9
- 10%AEP time of concentration = 10 minutes
- 10%AEP Developed Peak Flow = 1.529 m³/s
- 1%AEP time of concentration = 12 minutes
- 1%AEP Developed Peak Flow = 2.857 m³/s

From the Approved for Construction “1% AEP Drainage Outfall Detail Plan & Longsection” it can be seen that the 1%AEP developed peak flow was used to size the new outfall drainage network.

5.4 Proposed LPD

It is proposed to maintain the Hall Street table drain and new pipe network outfall to Stony Creek as the LPD's, as per the endorsed subdivision SMP.

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6 Stormwater Quantity Assessment

The aim of the stormwater quantity assessment is to demonstrate that the development limits the loading imposed on the existing Council drainage infrastructure, imposes no adverse effects on downstream properties or receiving water bodies and that the conveyance of flows will be in a safe manner with minimal risk of human endangerment.

The proposed drainage strategy was developed to maintain the catchment hydrology principles outlined within the endorsed subdivision SMP as well as the peak flow limitations at each LPD.

6.1 Proposed Development and Associated Issues

It is essential that there are no increases in volume and flow rate of stormwater runoff, and that any increases are mitigated such that post-developed peak flows do not exceed those for the pre-developed case. The proposed development leads to an overall decrease in impervious area compared to the sites previous condition, which implies there will be no increase in either discharge volume or flow rate.

6.2 Flow Rate Methodology

6.2.1 Design Storm Events

Based on recommendations within section 5.3.2 of the Melbourne Water Land Development Manual, AS/NZ 3500.3 and Council standards the major and minor storm events were selected as follows:

Minor Event: 10%AEP

- Surface drainage infrastructure sized for a 10%AEP storm through to point of discharge.

Major Event: 1%AEP

- Hydraulic roof water drainage to capture and connect flows up to and including the 1%AEP storm into the external civil drainage network.
- Surface drainage overflows in event of a 1%AEP storm to not present a hazard to people or cause significant damage to property.

Pipe sizing will be performed during detailed design and increased as required to ensure a safe depth vs velocity is maintained at all times during the major event.

6.2.2 Rational Method for Peak Flow Rate

The peak flow rate for the site has been obtained using the Rational Method. Summaries of the hydrology calculations can be seen in sections 6.4 and 6.5 for the pre and post-development scenarios respectively.

$$Q = (2.78 \times 10^{-3}) Cy Iy A$$

Equation 1

Q = Peak flow rate (m³/s) for average recurrence interval

Cy = Co-efficient of runoff for ARI of y years (dimensionless)

A = Catchment area (ha)

Iy = Average rainfall intensity (mm/hr) for a design duration of t hours and an ARI of y years

6.2.3 Catchment Area (A)

Catchment areas were measured using AutoCAD, contour surface data and known cadastral boundaries.

6.2.4 Time of Concentration

The time of concentration (tc) for each catchment was calculated using a combination of sheet flow, channel flow and standard inlet times.

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6.3 External Catchment

The grading of Hall Street and Simcock Avenue convey flows around the upstream extent of the site. Consequently, no upstream catchment is deemed to convey flows through the site.

6.4 Pre-Development Hydrology

The hydrology of the pre-developed catchment has been assessed in using the Rational Method. The theoretical calculated peak discharge for storm events ranging from the 100%AEP to the 1%AEP have been calculated and a summary of the results is presented in Table 2.

The subject site has a total area of 7.996 ha and prior to demolition comprised almost entirely of hardstand areas, as seen in Figure 3. The Coefficient of discharge ($C_{10\%}$) for each catchment was derived, and frequency factors were applied to determine runoff coefficients for various storm events.

Please refer to Table 2 for a summary of the Rational Method calculations and all parameters used.

Table 2 - Pre-development Catchment Details

Catchment I.D	Area (ha)	% Impervious	$C_{10\%}$	$C_{1\%}$	Time of Concentration (mins)	$Q_{10\%}$ (m ³ /s)	$Q_{1\%}$ (m ³ /s)
EX West	2.266	95%	0.87	1.00	12	0.419	0.808
EX East	5.730	95%	0.87	1.00	12	1.058	2.044
Total	7.996	95%	0.87	1.00	12	1.477	2.852

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6.5 Post-Development Hydrology

The total land area considered for the post development site was 7.996 ha. A catchment plan for the post-developed site can be found within Appendix C.

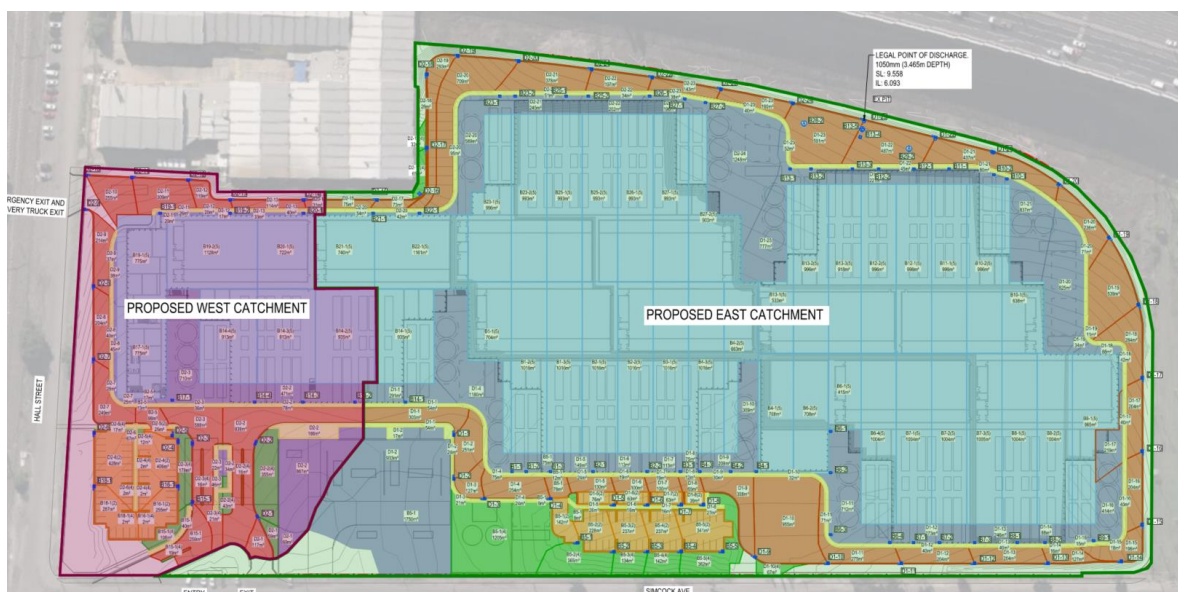


Figure 8 - Proposed 1%AEP overland catchments for post-development condition

The area and fraction impervious of the various catchments were determined. Subsequently, 10%AEP coefficients of runoff ($C_{10\%}$) values were adopted in accordance with section 5.3.2 of the Melbourne Water

Land Development Manual. F_y frequency factors were applied to determine runoff coefficients for various storm events.

Table 3 - Post-development Catchment Details

Catchment I.D	Area (ha)	% Impervious	$C_{10\%}$	$C_{1\%}$	Time of Concentration (mins)	$Q_{10\%}$ (m ³ /s)	$Q_{1\%}$ (m ³ /s)
PR West	1.749	80%	0.78	0.94	12	0.290	0.584
PR East	6.247	90%	0.84	1.00	12	1.114	2.229
Total	7.996	90.5%	0.84	1.00	12	1.404	2.813

As outlined in section 5.3.2, the endorsed subdivision SMP strategy involves reducing flows into the Hall Street table drain and increases flows draining north-east. However, as the proposal involves the construction of a new 1%AEP pipe network discharging directly to Stony Creek, the strategy is deemed to improve the local catchment hydrology and provide a safer discharge route into Stony Creek where all flows ultimately merge. A summary of the developed flows at each LPD are summarised below.

Hall Street Table Drain

- Western catchment gap flows (flows exceeding 10%AEP pipe network capacity) = 0.294 m³/s

New Pipe Network Outfall to Stony Creek

- Eastern catchment 1%AEP flow = 2.229 m³/s
- Eastern catchment 10%AEP flow = 1.114 m³/s
- Western catchment 10%AEP flow = 0.290 m³/s
- Total 10%AEP discharge to new pipe network outfall to Stony Creek = 1.404 m³/s
- Total 1%AEP discharge to new pipe network outfall to Stony Creek = 2.519 m³/s

It can therefore be seen that the proposed strategy reduces flows discharging to the Hall Street table drain and discharges flows within the capacity limitations of the new 1%AEP drainage outfall to Stony Creek.

6.5.1 Detention Analysis and Strategy

The proposed development leads to a decrease in impervious area compared to the pre-demolition scenario, and the results outlined in Section 6.4 and 6.5 demonstrate the decrease in total peak discharge from the subject site as a result of the proposed development. Thus, no detention measures are required for the development. This expectation was also outlined within Section 4.2 of the endorsed subdivision SMP.

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7 Stormwater Quality Assessment

7.1 Treatment Objectives

This assessment identifies issues relating to stormwater quality runoff and assesses possible methods of treatment if required. The aim of this section of the report is to determine practical approaches to achieving improvements in the quality of the stormwater run-off from the site that can be readily implemented and demonstrate compliance with VPP 2026 requirements.

The VPP 2026 specifies stormwater quality measures that must be implemented for any new development. Clause 53.18-5 of the VPP states that the stormwater management objectives for buildings and works are to:

- Meet the current best practice performance objectives for stormwater quality as contained in the *Urban Stormwater – Best Practice Environmental Management Guidelines* (Victorian Stormwater Committee, 1999);
- Minimise the impact of chemical pollutants and other toxicants including by, but not limited to, bunding and covering or roofing of storage, loading and work areas; and
- Contribute to cooling, improving local habitat and providing attractive and enjoyable spaces.

Table 2.1 of *Urban Stormwater – Best Practice Environmental Management Guidelines* suggests 'Post Construction Phase – Current Best Practice Performance Objectives' as:

Table 4 - Objectives for Environmental Management of Stormwater

Total Suspended Solids (TSS)	Total Phosphorus (TP)	Total Nitrogen (TN)	Gross Pollutants >5mm
80% Retention	45% Retention	45% Retention	70% Reduction

The objective is to provide the following:

- Nitrogen and Phosphorous removal
- Gross Pollutant and Suspended Solids Removal
- All of the site's impervious areas discharge to suitable treatment device/s
- Treatment device selection criteria are to be in accordance with Industry Best Practice and, WSUD Engineering Guidelines
- Provide engineering diagrams of the stormwater quality treatment of the proposed development

7.2 Erosion and Sediment Control

7.2.1 Pre-Development Phase

Prior to construction commencing, the following erosion and sediment control measures will need to be installed around the subject site to minimise disturbance and ensure the quality of runoff discharging from the site is of an acceptable standard:

- Sediment barriers to be installed on all entrances to downstream stormwater infrastructure (i.e. gully pits);
- Designation of transport routes through the site to minimise vegetation disturbance;
- Maximise retention of existing vegetation to reduce soil disturbance and provide filter strip treatment for runoff;
- Install construction entry and exit shakedown areas;
- Sediment fences are to be installed on the downstream boundaries of the subject site; and
- Install dust control measures as required.

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All erosion and sediment control measures are to be designed and installed in accordance with IECA Guidelines.

7.2.2 Bulk-Earthworks Phase

During the bulk earthworks phase, the following erosion and sediment control measure will need to be installed in addition to the aforementioned measures (Pre-Development Phase) to ensure there is minimal disturbance to downstream receiving water bodies:

- Construction chutes to control runoff over earthworks batters;
- Construction of temporary bunds at the top of all earthworks batters to ensure runoff is directed away from exposed batters;
- Sediment basins to be constructed at low points within each stage of the proposed development;
- Construction of temporary diversion drains to divert water to sediment basins and around any stockpiles;
- Sediment fences to be installed on the downstream side of any stockpiles; and
- Stabilisation of all batters upon reaching the finished earthworks levels.

All erosion and sediment control measures are to be designed and installed in accordance with IECA Guidelines.

7.2.3 Construction Phase

During the construction phase of the development, there is a risk of sedimentation transport due to large areas of disturbed land. The following erosion and sediment control measures will need to be installed in addition to the aforementioned measures (Pre-Development and Bulk Earthworks Phases) to ensure there is minimal disturbance, and the quality of runoff is maintained to an acceptable standard:

- Construction of temporary diversion drains to divert water to sediment basins;
- Construction of temporary diversion drains to divert water to protect stormwater treatment devices as required;
- Sediment barriers to be installed on all entrances to newly constructed stormwater infrastructure (i.e. gully pits);
- Sediment fences to be installed on the downstream side of any stockpiles and batters; and
- Re-vegetation of all disturbed areas within two (2) weeks of completion.

All erosion and sediment control measures are to be designed and installed in accordance with IECA Guidelines.

7.2.4 Maintenance

All erosion and sediment control devices are to be maintained throughout the entire phase of the development leading up to the operational phase. Erosion and sediment control devices will need to be monitored closely throughout the entire project to ensure they are operating correctly and efficiently.

7.3 Operational Phase Treatment

During the operational phase, it is proposed to have roof, road and internal landscaped areas draining through OceanGuard trash baskets fitted within inlet pits before passing through a Cascade Separator before finally discharging through an OceanSave GPT to the LPD. Some areas of boundary landscaping fall directly outwards and so were modelled as bypassing catchments.

Internal stormwater drainage shall be designed and constructed in accordance with AS3500.3 and all other relevant standards and guidelines.

7.4 Stormwater Quality Improvement Devices (SQIDs)

The proposed stormwater quality treatment measures for the development will consist of:

- 2x Ocean Protect CS2250 Cascade Separator's
- 1x Ocean Protect OS-2318 OceanSave Gross Pollutant Trap
- Ocean Protect OceanGuard Trash Baskets fitted within Inlet Pits

7.4.1 OceanSave

OceanSave's are high-capacity gross pollutant traps designed to remove particles greater than 1mm and separate and trap litter, sediment and hydrocarbons.

7.4.2 Cascade Separator

Cascade Separator's capture and retain sediment while also removing hydrocarbons, trash and debris.

7.4.3 OceanGuard

OceanGuard's are trash baskets designed to capture pollution that runs into stormwater drains and can be fitted within new and existing stormwater pits.

7.5 MUSIC Model

The sites stormwater run-off was modelled using MUSIC (version 6.3.0) and the water quality objectives for Victoria specified in the VPP 2020 of 80% TS retention, 45% TP retention, 45% TN retention, and 90% Gross Pollutants reduction.

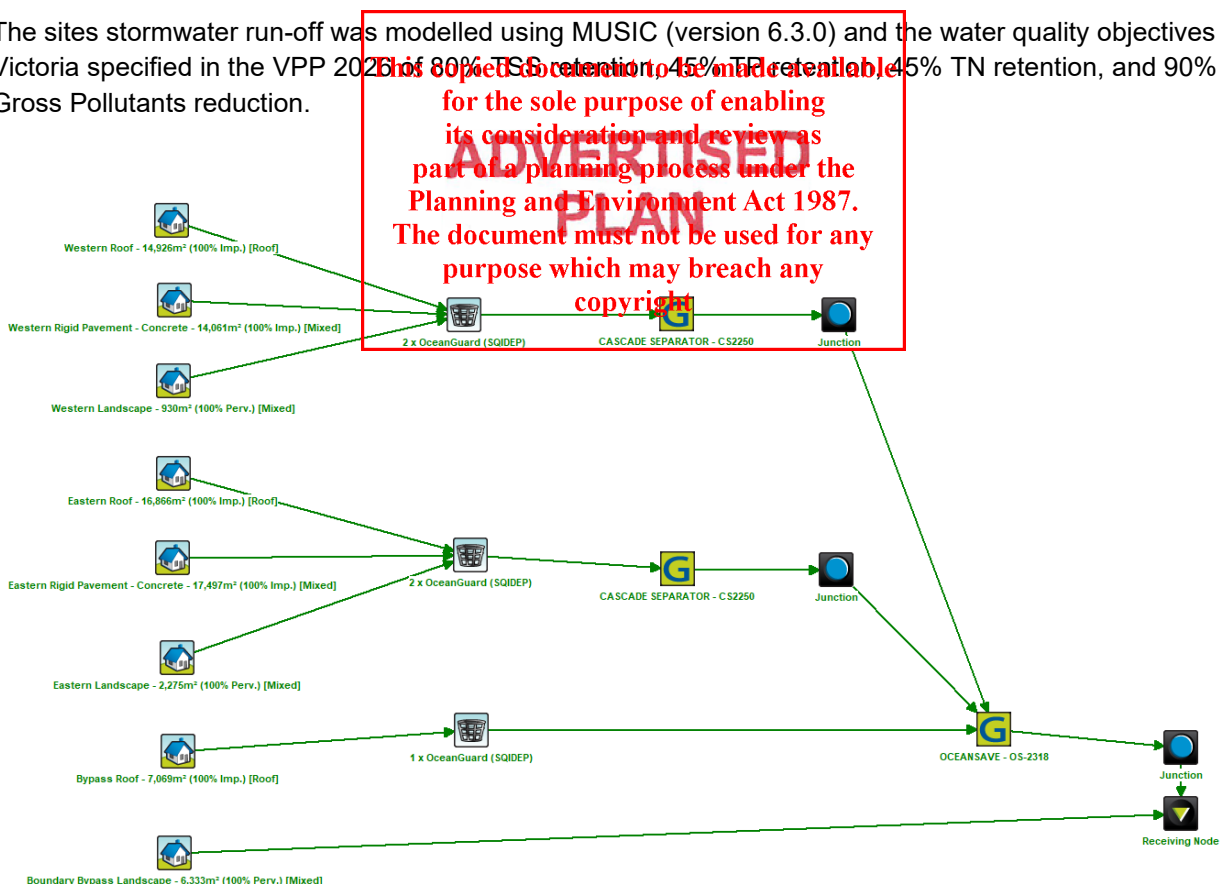


Figure 9 - Treatment Train

The results of the above MUSIC model are presented in Figure 10.

	Sources	Residual Load	% Reduction
Flow (ML/yr)	33	33	0
Total Suspended Solids (kg/yr)	5730	320	94.4
Total Phosphorus (kg/yr)	11.3	3.06	73
Total Nitrogen (kg/yr)	81	42.9	47.1
Gross Pollutants (kg/yr)	1250	2.71	99.8

Figure 10 - Results for the treatment train

The above results meet the percent reduction water quality objectives identified by the Hobsons Bay Planning Scheme.

Details of the MUSIC model are attached within Appendix F for further information.

7.6 On-site Treatment Lifecycle Costs

A lifecycle cost analysis is not a part of the scope of this report. All the recommended water quality treatment infrastructure lies within the development site, and it shall be maintained and serviced by the owners of the development at no cost to Council.

7.7 Water Quality Monitoring

No water quality monitoring is proposed for this development at this stage due to the nature of the development and the expected pollutant levels.

7.8 Maintenance

Maintenance of the SQIDs will be the responsibility of the owners of the development. The maintenance should be carried out in accordance with the manufacturer's recommendations.

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8 Conclusion

As outlined in Section 6 of this report, the proposed development results in a decrease to the total impervious areas. Thus, the post-developed flows are less than the previous flows when the site was used as an oil terminal. Consequently, no stormwater detention measures have been proposed. This expectation was also outlined within Section 4.2 of the endorsed subdivision SMP. Developed flows will discharge to the proposed legal points of discharge nominated within the endorsed subdivision Stormwater Management Plan.

This report demonstrates a pollutant load reduction as required by the VPP 2026. Aurecon recommends the use of the following treatment devices to meet the treatment targets specified by the relevant authorities:

- 2x Ocean Protect CS2250 Cascade Separator's
- 1x Ocean Protect OS-2318 OceanSave Gross Pollutant Trap
- Ocean Protect OceanGuard Trash Baskets fitted within Inlet Pits

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Appendix A – Existing Conditions Survey Plan

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Appendix B – Architectural Drawings

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Appendix C – Civil Drawings

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Appendix D – Endorsed Subdivision Stormwater Management Plan

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Appendix E – Planning Property Reports

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Appendix F – MUSIC Model Information

Introduction:

The quality of stormwater runoff and the impact of the proposed stormwater quality improvement measures were analysed using MUSIC Version 6.3.0 according to the Melbourne Water MUSIC Guidelines 2024. A summary of the modelled catchment is presented in the table below.

Catchment I.D	Land Use	Area (m ²)	% Impervious
Western Roof	Roof	14,926	100%
Western Rigid Pavement - Concrete	Mixed	14,061	100%
Western Landscape	Mixed	930	0%
Eastern Roof	Roof	16,866	100%
Eastern Rigid Pavement - Concrete	Mixed	17,497	100%
Eastern Landscape	Mixed	2,275	0%
Bypass Roof	Roof	7,069	100%
Boundary Bypass Landscape	Mixed	6,333	0%

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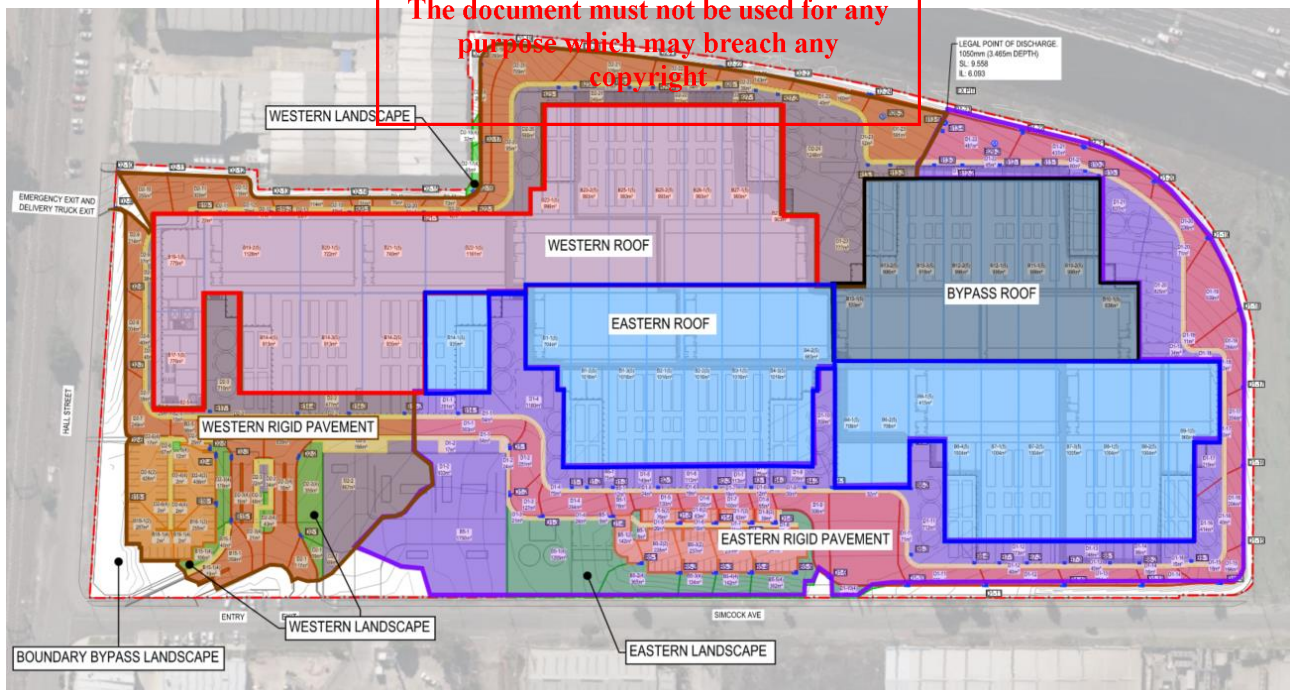


Figure 11 - Layout of MUSIC Catchments

Meteorological Data:

The MUSIC model was carried out using the following parameters:

- The modelling period should be 10 years with a time step of 6 minutes.
- The nearest available 6-minute time step rainfall series to the subject site is Melbourne Airport.

Meteorological Data Statistics

	Rainfall/6 Minutes	Evapo-Transpiration
mean	0.007	2.865
median	0.000	2.170
maximum	9.560	5.000
minimum	0.000	1.000
10 percentile	0.000	1.130
90 percentile	0.000	4.520
mean annual	574	1047

Close Print...

Catchment Properties

Catchment Name	260106_Sitewide DA
Rainfall Station	086282 MELBOURNE
ET Station	User-defined monthly PET
Start Date	1/01/1987 12:00 AM
End Date	31/12/1996 11:54 PM
Modelling Time Step	6 Minutes

Close

Source Nodes – Pollutant Concentration Data:

Pollutant concentration parameters were assigned as per Table 4 of the Melbourne Water MUSIC Guidelines.

The pollutant exports parameters adopted in the MUSIC model are summarized in the table below.

Pollutant	Surface type	Storm flow (log mg/L)		Baseflow (log mg/L)	
		Mean	SD	Mean	SD
TSS	Roof	1.301	0.333	n/a*	n/a
	Road	2.431	0.333	n/a	n/a
	All other urban areas	1.882	0.333	0.96	0.401
TP	Roof	-0.886	0.242	n/a	n/a
	Road	-0.301	0.242	n/a	n/a
	All other urban areas	-0.680	0.242	-0.731	0.360
TN	Roof	0.301	0.205	n/a	n/a
	Road	0.342	0.205	n/a	n/a
	All other urban areas	0.224	0.205	0.346	0.309

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Input Parameters:

Ocean Guards:

Properties of 2 x OceanGuard (SQIDEP)

Location: 2 x OceanGuard (SQIDEP)  Products >>

Inlet Properties

Low Flow By-pass (cubic metres per sec): 0.00000

High Flow By-pass (cubic metres per sec): 0.04000

Target Element

☒ Gross Pollutants (kg/ML) ☐ Total Phosphorus (mg/L)

☐ Total Suspended Solids (mg/L) ☐ Total Nitrogen (mg/L)

Gross Pollutants (kg/ML)

Transfer Functions

☒ Concentration Based Capture Efficiency ☐ Flow Based Capture Efficiency

☐ Both

Concentration Based Capture Efficiency

Input	Output
0.0000	0.0000
1000.0000	0.0000

Flow Based Capture Efficiency

Inflow (m ³ /s)	% Capture
0.0000	100.0000
1.0000	100.0000

Fluxes... Notes...

 Cancel  Back  Finish

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Cascade Separator:

Properties of CASCADE SEPARATOR - CS2250

Location: CASCADE SEPARATOR - CS2250

Inlet Properties

Low Flow By-pass (cubic metres per sec): 0.00000

High Flow By-pass (cubic metres per sec): 0.17400

Target Element

☒ Flow (cubic metres per sec)

☐ Gross Pollutants (kg/ML)

☐ Total Suspended Solids (mg/L)

☐ Total Phosphorus (mg/L)

☐ Total Nitrogen (mg/L)

Flow (cubic metres per sec)

Transfer Functions

☒ Concentration Based Capture Efficiency

☐ Flow Based Capture Efficiency

☐ Both

Concentration Based Capture Efficiency

Inflow	Outflow
0.0000	0.0000
10.0000	10.0000

Flow Based Capture Efficiency

Inflow (m ³ /s)	% Capture
----------------------------	-----------

Fluxes...

Notes...

Cancel Back Finish

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OceanSave:

Properties of OCEANSAVE - OS-2318

Location: OCEANSAVE - OS-2318

Inlet Properties

Low Flow By-pass (cubic metres per sec): 0.00000

High Flow By-pass (cubic metres per sec): 0.58000

Target Element

☒ Flow (cubic metres per sec) ☐ Total Phosphorus (mg/L)

☐ Gross Pollutants (kg/ML) ☐ Total Nitrogen (mg/L)

☐ Total Suspended Solids (mg/L)

Flow (cubic metres per sec)

Transfer Functions

☒ Concentration Based Capture Efficiency ☐ Flow Based Capture Efficiency

☐ Both

Concentration Based Capture Efficiency

Inflow	Outflow
0.0000	0.0000
10.0000	10.0000

Flow Based Capture Efficiency

Inflow (m ³ /s)	% Capture
----------------------------	-----------

Fluxes... Notes...

Cancel Back Finish

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Appendix G – SQID Maintenance

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